

Velocity defect law in the wave bottom boundary layer

Objective

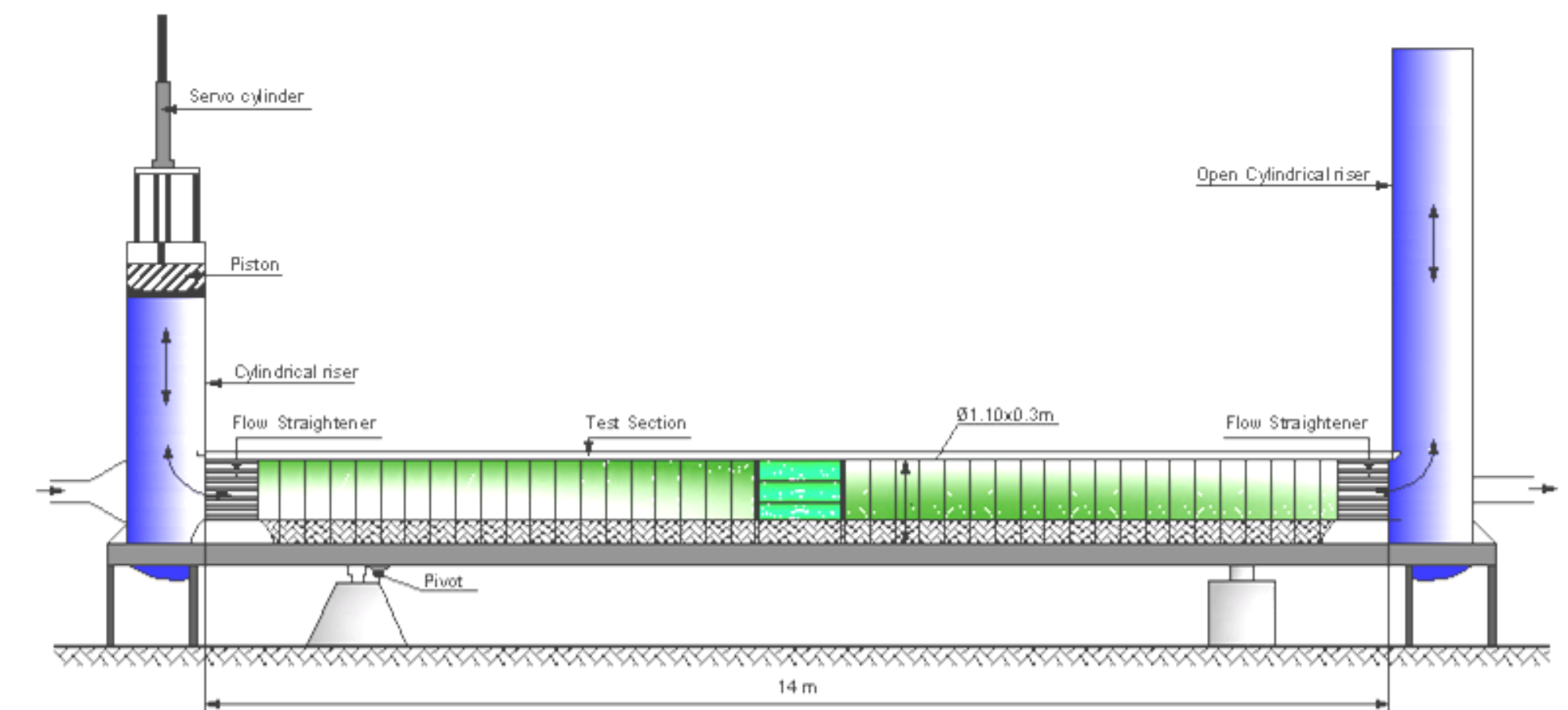
This work presents a simple method based on the defect law (Nielsen, 1992) to reproduce the velocity vertical profile within the wave bottom boundary layer.

TRANSKEW experiment

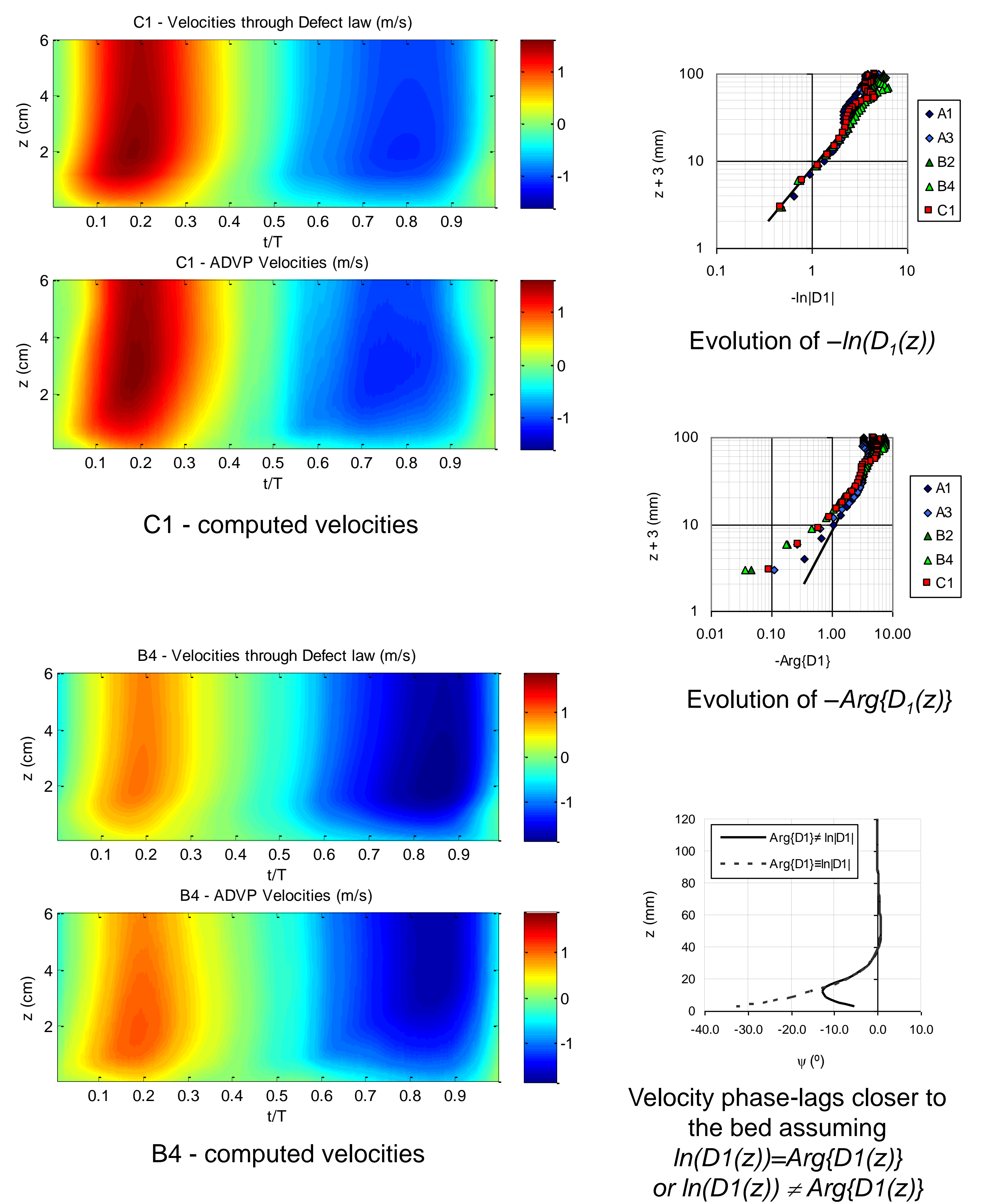
- New series of experiments to evaluate the net transport rates in **sheet flow regime**, (well-sorted sand bed, $d_{50} \approx 0.20$ mm), under accelerated skewed waves, (Silva *et al.*, 2010).
- Different hydraulic conditions
 - **Series A:** regular oscillatory flows with different degrees of acceleration skewness, β ;
 - **Series B:** acceleration-skewed oscillatory flows with a collinear net current, opposing the wave direction;
 - **Series C:** velocity- and acceleration- skewed oscillatory flows.
- An Acoustic Doppler Velocity Profiler (ADVP) measured simultaneously both horizontal and vertical velocities every 3mm over a 14cm layer immediately above the bed.

Condition	β^a	R^b	T^c [s]	U_0^d [m/s]
A1	0.65	0.5	7	0
A2	0.65	0.5	10	0
A3	0.75	0.5	7	0
A4	0.75	0.5	10	0
B1	0.65	0.5	7	-0.2
B2	0.65	0.5	7	-0.4
B3	0.75	0.5	7	-0.2
B4	0.75	0.5	7	-0.4
C1	0.65	0.6	7	0
C2	0.65	0.6	10	0
C3	0.50	0.6	7	0

^a β is acceleration skewness, $a_{\max}/(a_{\max} - a_{\min})$, where a is acceleration
^b R is velocity skewness, $u_{\max}/(u_{\max} - u_{\min})$, where u is velocity
^c T is wave period
^d U_0 is net current



Results



Analysis Procedure

DEFECT LAW

- the velocities $u(z,t)$ inside the wave bottom boundary layer can be written in terms of the free stream velocity, $u_{\infty}(t)$, and a dimensionless velocity defect function $D_1(z)$:

$$u(z,t) = [1 - D_1(z)] u_{\infty}(t) \quad (1)$$

- Nielsen (1992) suggested that, for turbulent flows, $D_1(z)$ requires the knowledge of a vertical scale, z_1 , and a power p that fits the data:

$$-\ln|D_1(z)| = \left(\frac{z}{z_1}\right)^p \quad (2)$$



- An analysis of the primary harmonic of the velocity records from ADVP pointed $z_1 \approx 8\text{mm}$ and $p \approx 0.75$ for all the experiments.

- Abreu *et al.* (2010) showed that an arbitrary nonlinear free stream velocity, $u_{\infty}(t)$, can be represented according to 4 parameters (U_w, T, r, ϕ):

$$u_{\infty}(t) = U_w \sqrt{1-r^2} \frac{\left[\sin(\omega t) + \frac{r \sin \phi}{1 + \sqrt{1-r^2}} \right]}{[1 - r \cos(\omega t + \phi)]} \quad (3)$$

- Eq. (2) was combined with Eq. (3) to reproduce $u(z,t)$ inside the wave bottom boundary layer.

Main Conclusions

- The model results agree fairly well with the ADVP measurements and show that the defect law reproduces typical features of the oscillatory boundary layer: the velocity magnitude first increases with distance from the bed, with an overshoot at approximately 3 cm above the bed.
- There is a phase shift in the velocity that is maximum at about 1cm above the bed.
- Processing of the bed shear stress as well as velocities estimates within the sheet flow layer is under progress. (e.g. Ruessink *et al.*, submitted)

REFERENCES

- Abreu, T., Silva, P.A., Sancho, F. and Temperville, A. (2010). Analytical approximate wave form for asymmetric waves. *Coastal Engineering*, 57, 656-667.
- Nielsen, P., 1992. *Coastal Bottom Boundary Layers and Sediment Transport*. World Scientific, 324 pp.
- Silva, P.A., Abreu, T., Van der A, D.A., Sancho, F., Ruessink, B.G., Van der Werf, J.J. and Ribberink, J.S. (Submitted). Sediment transport in non-linear skewed oscillatory flows: the Transkew experiments. *Journal of Hydraulic Research*.
- B.G., Ruessink, Michallet, H., Abreu, T., Sancho, F., Van der A, D.A., Van der Werf, J.J. and Silva, P.A. (Submitted). Observations of velocities, sand concentrations, and fluxes under monochromatic velocity-asymmetric oscillatory flows. *Journal of Geophysical Research*.

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