# Study of Primary and Internal Resonance on 3D Free-Free Double-Section Beam

Yi-Ren Wang<sup>\*</sup>, Yun-Shuo Chang

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## Abstract

This work investigates the primary resonance and internal resonance of a double-section beam with cubic nonlinearities. This model can be applied in a wide range of engineering problems, such as rocket and missile structures. Even space technology has been developed for decades; several nonlinear properties deserve further study, especially, for the internal resonance. The method of multiple scales (a perturbation technique) is employed to analyze this nonlinear problem. This study focuses on finding the forcing conditions of this 3D double-section beam to trigger the often-ignored internal resonance or prime resonance in rocket structures. A primary resonance is found on a uniform free-free beam at certain flight speed. The three-to-one internal resonance of the double-section beam occurs within the first and the second modes in the diameter ratio of 1/0.75 with the length ratio of 0.33 or 0.51. The semi-analytical results are verified by the time marching numerical method.

Keywords: nonlinear vibration, internal resonance, method of multiple scales

## 1. Introduction

Studies of vibration have always been a concern for researchers and engineers because it may cause the structural fatigue or failure. Beams are widely applied in engineering, such as wings in aerospace, bridges in civil engineering, and train rails in mechanical engineering. Many studies on beam vibrations have been performed previously. Özkaya [1] researched on a beam-mass system under simply supported end conditions, and the effect of positions, magnitudes, and the number of the masses was investigated. Mundrey [2] considered the 2D Bernoulli-Euler beam resting on an elastic foundation to simulate the railway track and a moving load on the 2D beam was studied. Phuoc Nguyen et al. [3] considered the dynamic response of the Euler-Bernoulli beam subjected to moving oscillators. Chang [4] used a variational method to study nonlinear vibrations in carbon nanobeams under the magnetic field. Zhang et al. [5] provided a controllable active torque actuator model for wind-turbine tower vibrations and demonstrated that the proposed module can effectively mitigate the vibrations in wind turbines during operation. These studies demonstrated a wide application of nonlinear models; yet, a 3D nonlinear model and some unique nonlinear properties should be discussed further.

In most nonlinear beam problems, an internal resonance is a major point of discussion. The internal resonance is unique to nonlinear systems in which integer relationships exist among the natural frequencies with various modes. Due to nonlinearity, the internal resonance generally occurs in modes that are not being directly excited by external forces. Exciting the higher modes can lead to high amplitude vibrations in lower modes [6]. Large vibration amplitude of the unexcited mode of nonlinear beams could be triggered due to internal resonance. Therefore, the lower modes should not be overlooked. However, it is interesting that in common 3D beams with symmetrical cross sections; one-to-one internal resonance is the most likely to take

<sup>\*</sup> Corresponding author. E-mail address: 090730@mail.tku.edu.tw

Tel.: +886-2-26215656; Fax: +886-2-26209746

place among the various degrees of freedom. As its resonant frequency is the same as each other, it is also called primary resonance. For instance, Pai [7] analyzed the primary resonance in a 3D nonlinear composite rotating beam. Stoykov and Ribeiro [8] examined the stability of a 3D nonlinear rotating beam based on Timoshenko's theory and took into account the deformation caused by twists and warps. Research has also been conducted on the internal resonance of beams caused by external forces or additional structures, such as those associated with the suspension or support. For instance, van Horssen et al. [9-10] considered the use of nonlinear aerodynamics to create three-to-one internal resonance in systems with elastic foundations or suspension springs. Wang et al. [11] analyzed a hinged-hinged nonlinear Bernoulli-Euler beam with linear and nonlinear tuned dampers. The internal resonance was investigated for the system. The nonlinear multi-loaded slender beam and the nonlinear primary resonance was first found in the wind turbine tower. Tekin et al. [13] considered the three-to-one internal resonance in the multiple stepped beam systems. These previous researches investigated the nonlinear properties of internal resonance and primary resonance. However, the internal and primary resonance conditions might happen simultaneously for a 3D beam. They should be taken into consideration at the same time.

In addressing general analytical methods to nonlinear vibrations, Nayfeh and Pai [14] investigated vibrations in nonlinear Euler-Bernoulli beams. They formulated a number of useful nonlinear beams equations in accordance with Newton's laws, Euler angles transformation, and the Karman-type strain-displacement relationship. Nayfeh and Mook [6] also proposed a number of perturbation methods by which to solve nonlinear systems, including the Lindstedt-Poincaré method, the method of averaging, and the method of multiple scales (MOMS). Perturbation methods allow the researchers to get good approximations for systems that the exact solutions are not all easy to be solved. Ji and Zu [15] studied the rotating shaft system of a Timoshenko beam, using MOMS to analyze the natural frequency responses of nonlinear systems. Nayfeh and Nayfeh [16] employed MOMS to identify nonlinear modes and nonlinear frequencies, whereupon they applied the Galerkin method to the analysis of dynamic responses in a nonlinear beam. Mao [17] used the ADM method to analyze the vibration of beams consisting of an arbitrary number of steps through a recursive method. He showed that the ADM offers an accurate and effective method of free vibration analysis of multiple-stepped beams with arbitrary boundary conditions.

The 3D free-free beam model can be used to simulate space rocket vibration in high-speed motion. The double-section 3D beam model can also be utilized to model a high-speed double-stage rocket motion. The most concerns on the multi-stage rocket motion are the stage separation dynamic control [18] or the high speed flows over the joint of the space rocket stages [19].

According to the references aforementioned with 3D beams or rocket motions, the nonlinear properties of both primary resonance and internal resonance are less discussed simultaneously in the 3D beams or rocket structures. The present study considers a nonlinear double section free-free beam subjected to the distributed load with the wind force and its associated unsteady aerodynamic force. Since the primary resonance or internal resonance is unique in nonlinear problems, the unexcited modes usually have larger amplitudes than the excited modes. However, these conditions cannot be predicted by using linear theories. It is worth having a deep study on this model.

The present work focuses on finding the forcing conditions of this 3D double-section beam to trigger the often-ignored internal resonance or prime resonance in rocket structures. This research uses Newton's second law to derive the nonlinear equation of motion. The method of multiple scales (MOMS) is used to obtain the steady state frequency response (fixed point). A primary resonance is found for a uniform free-free beam at certain flight speed. The three-to-one internal resonance is also triggered within the first and the second modes in certain combinations of beam length and diameter ratios of a double-section beam. The fixed points plot is used to observe the nonlinear phenomenon in the system. The fourth-order Runge-Kutta method is also performed to verify the results of frequency response and the internal resonance.

## 2. Theoretical Model

#### 2.1. Equations of motion for uniform free-free beam

This study analyzes the vibrations in a straight 3D free-free nonlinear beam, which could be used to simulate the high speed aircrafts such as rocket, missiles, etc. The schematic of the space rocket, the coordinate definitions of the simulated beam, and the relationships between the various external forces are demonstrated in Fig. 1. Fig. 1(a) shows the space rocket model similar to the Satellite and Rocket Propulsion Lab. at NCKU. It is true that the proposed beam model can be applied to simulate a missile or a rocket vibration motion. Even though the rocket model is hollow, the bulkheads, formers, longerons, stringers and struts strengthen the fuselage structure of the rocket. The beam model is used to simulate the rocket motion for a preliminary study. In this research, the internal resonance and the primary resonance of the rocket nonlinear properties are studied. Fig. 1(b) represents the schematic of the coordinate definitions and the multiple forces applied to a uniform free-free beam. The initial status of the beam is considered straight under the assumption that each cross-section is a plane. The deformed of the beam also follows the stress-strain laws. Three consecutive Euler angles are used to relate the deformed and unreformed states. The equations contain structural coupling terms, quadratic, and cubic nonlinearities due to curvature and inertia. Based on Newton's 2<sup>nd</sup> law, Euler angle transformation, and Taylor series expansion, the equations of motion of the nonlinear beam can be expressed as follows [14]:

$$m\ddot{v} + \overline{c}_y \dot{v} = G'_y + F_y \tag{1}$$

$$m\ddot{w} + \bar{c}_{\dot{w}} = G' + \bar{F}_{\dot{w}}$$

$$\overline{j}_x \dot{\gamma} + \overline{c}_x \dot{\gamma} = G_x + \overline{M}_x \tag{3}$$



(a) Schematic of NCKU HTPB/N<sub>2</sub>O hybrid rocket model



(b) Schematic of multiple forces applied to a uniform free-free beam

Fig. 1 The rocket model and a uniform free-free beam approximated model

where *m* is the beam mass per unit length,  $\bar{J}_x$  is the mass moment of inertia in the *x*-direction,  $\bar{c}_y$ ,  $\bar{c}_z$ , and  $\bar{c}_x$  represent the damping coefficient of the beam in the *y*-, *z*-, and *x*-axis, respectively. The  $\bar{F}_y$ ,  $\bar{F}_z$ , and  $\bar{M}_x$  represent the unsteady harmonic wind force and aerodynamic moment in the *y*-, *z*-, and *x*-directions, respectively. The *v*, *w*, and  $\gamma$  represent the beam displacement (or twist angle) in the *y*-, *z*- and *x*-directions, respectively. (·) denotes  $d/d\bar{t}$ , and ()' denotes  $d/d\bar{x}$ . Furthermore,  $G_v$ ,  $G_z$ , and  $G_x$  are defined below for a uniform and isotropic beam [14]:

$$G_{y} = -(D_{zz}v'')' - D_{xx}(\gamma'w'') - D_{zz}v'(v'v'' + w'w'')' + (D_{yy} - D_{zz})[(w''\gamma - v''\gamma^{2})' - w''']_{0}^{x}v''w'd\overline{x}] - (1/2)v'\int_{\overline{t}}^{\overline{x}} m[\int_{0}^{\overline{x}} (v'^{2} + w'^{2})d\overline{x}] \cdot d\overline{x}$$

$$(4)$$

$$G_{z} = -(D_{yy}w'')' + D_{xx}(\gamma'v'') - D_{yy}w'(v'v'' + w'w'')' + (D_{yy} - D_{zz})[(v''\gamma + w''\gamma^{2})' + v'''\int_{0}^{\overline{x}}w''v'd\overline{x}] - (1/2)w'\int_{\overline{t}}^{\overline{x}}m[\int_{0}^{\overline{x}}(v'^{2} + w'^{2})d\overline{x}] \cdot d\overline{x}$$
(5)

$$G_{x} = (D_{xx}\gamma')' + (D_{zz} - D_{yy})[(v''^{2} - w''^{2})\gamma - v''w''] + \overline{j}_{x}[(\int_{0}^{\overline{x}} v''w'd\overline{x})'' - (\dot{v}'w')'] + (\overline{j}_{y} - \overline{j}_{z})[(\dot{v}'^{2} - \dot{w}'^{2})\gamma - \dot{w}'\dot{v}']$$
(6)

where  $\bar{x}$  is the position coordinate along the beam in the axial direction,  $\bar{l}$  is the beam length,  $D_{xx} = G\bar{l}_x$ ,  $D_{yy} = E\bar{l}_y$ ,  $D_{zz} = E\bar{l}_z$ , and *G* is the elastic shear modulus and *E* is Young's modulus. The  $\bar{l}_{x,y,z}$  represent the area moment of inertia in the *x*-, *y*-, and *z*-directions, respectively. The dimensionless coefficients are defined as follows:

$$\begin{cases} l = \overline{l} / \overline{l} = 1, \ x = \overline{x} / \overline{l}, \ t = \overline{t} \sqrt{D_{yy} / m \overline{l}^4}, \ c_y = \overline{c}_y \overline{l}^2 / \sqrt{m D_{yy}} \\ c_z = \overline{c}_z \overline{l}^2 / \sqrt{m D_{yy}}, \ c_x = \overline{c}_x / \sqrt{m D_{yy}} j_x, \ j_x = \overline{j}_x / m \overline{l}^2 \\ j_y = \overline{j}_y / m \overline{l}^2, \ j_z = \overline{j}_z / m \overline{l}^2, \ \mu_{xy} = D_{xx} / D_{yy}, \ \mu_{zy} = D_{zz} / D_{yy} \end{cases}$$
(7)

To simplify the notation, the symbols for the dimensionless displacement of the beam were used as the same as the three axes. In other words, y and z are respectively used to represent the transverse (transverse dir.,  $y = v/\bar{l}$ ) and lateral (side dir.,  $z = w/\bar{l}$ ) dimensionless displacement functions to avoid influencing the derivation of the theoretical model and to maintain consistency in the overall equations of motion. Therefore, the 3D flexural-flexural-torsional vibration of uniform isotropic beam equation can be written as:

$$\ddot{y} + \mu_{zy} y^{iv} = -c_y \dot{y} - \mu_{xy} (\gamma' z'') - \mu_{zy} [y'(y'y'' + z'z'')'] - (1 - \mu_{zy}) [(y'' \gamma^2 - z'' \gamma)' + z''' \int_0^x y'' z' dx]' - \frac{1}{2} \{y' \int_1^x [\int_0^x (y'^2 + z'^2) dx] dx' + F_y$$
(8)

$$\ddot{z} + z^{iv} = -c_z \dot{z} + \mu_{xy} (\gamma' y'')' - [z'(y'y'' + z'z'')']' + (1 - \mu_{zy})[(y''\gamma + z''\gamma^2)' + y''']_0^x y'z''dx]' - \frac{1}{2} \{z' \int_1^x [\int_0^x (y'^2 + z'^2) dx] \, \dot{d}x\}' + F_z$$
<sup>(9)</sup>

$$\ddot{\gamma} - \frac{\mu_{xy}}{j_x} \gamma'' = -c_x \dot{\gamma} + \frac{\mu_{zy} - 1}{j_x} (y''^2 \gamma - z''^2 \gamma - y''z'') + (\int_0^x y''z' dx) - (\dot{y}z') + \frac{j_y - j_z}{j_x} (\dot{y}'^2 \gamma - \dot{z}'^2 \gamma - \dot{y}\dot{z}') + M_x$$
(10)

In addition, the cross-section of the beam in this study is a circle, and the ratio of the lateral area moments of inertia is equal to 1, which means that  $\mu_{zy} = 1$ . The structural damping coefficients are also the same, which means that  $c_y = c_z$ . It is noted that the structural damping is always used to simulate a solid material when it flexes. As for the simulation of beam vibrations, the damping coefficient is rather small, and  $c_y$  is taken as 0.1 in this study.

Assuming the beam is subjected to a distributed load with the harmonic wind force  $\bar{q}_{y,z}e^{i\bar{\Omega}_{y,z}\bar{t}}$ . Its associate unsteady aerodynamic force  $\bar{F}_{D,A}$ , that is,  $\bar{F}_{y,z}$  can be expressed as  $\bar{F}_{y,z} = \bar{q}_{y,z}e^{i\bar{\Omega}_{y,z}\bar{t}} + \bar{F}_{D,A}$ . The wind force can be normalized as:

$$q_{y,z} = \overline{q}_{y,z} / D_{yy} \overline{l}$$

$$\Omega_{y,z} = \overline{\Omega}_{y,z} \overline{l}^2 \sqrt{D_{yy} / m \overline{l}^4}$$
(11)

The windward aerodynamic forces in the *y*-direction of the beam are given as follows (please refer the research from van Horssen [20]),

$$\begin{cases} \overline{F}_{D} = \frac{\rho_{a}\overline{U}_{y}^{2}\overline{d}}{2} \left( a_{D0U} + \frac{a_{D1U}}{\overline{U}_{y}} \frac{\partial \overline{y}(\overline{x},\overline{t})}{\partial \overline{t}} + \frac{a_{D2U}}{\overline{U}_{y}^{2}} \frac{\partial \overline{y}^{2}(\overline{x},\overline{t})}{\partial \overline{t}} + \frac{a_{D3U}}{\overline{U}_{y}^{3}} \frac{\partial \overline{y}^{3}(\overline{x},\overline{t})}{\partial \overline{t}} + \dots \right) \\ \overline{F}_{A} = \frac{\rho_{a}\overline{U}_{z}^{2}\overline{d}}{2} \left( a_{A0U} + \frac{a_{A1U}}{\overline{U}_{z}} \frac{\partial \overline{z}(\overline{x},\overline{t})}{\partial \overline{t}} + \frac{a_{A2U}}{\overline{U}_{z}^{2}} \frac{\partial \overline{z}^{2}(\overline{x},\overline{t})}{\partial \overline{t}} + \frac{a_{A3U}}{\overline{U}_{z}^{3}} \frac{\partial \overline{z}^{3}(\overline{x},\overline{t})}{\partial \overline{t}} + \dots \right) \end{cases}$$
(12)

Normalizing Eq. (12) yields:

$$\begin{cases} F_D = \hat{a}_{D0U} + \hat{a}_{D1U} \dot{y} + \hat{a}_{D2U} \dot{y}^2 + \hat{a}_{D3U} \dot{y}^3 + ..., \\ F_A = \hat{a}_{A0U} + \hat{a}_{A1U} \dot{z} + \hat{a}_{A2U} \dot{z}^2 + \hat{a}_{A3U} \dot{z}^3 + ..., \end{cases}$$
(13)

where  $\hat{a}_{D0U}$  is the lifting coefficients and  $\hat{a}_{D1U}$  is the first-order derivative modified term. Both of them are defined as:

$$\hat{a}_{D0U} = \frac{\rho_a \bar{U}_y^2 \bar{d} a_{D0U}}{2m\omega^2} \tag{14}$$

$$\hat{a}_{D1U} = \frac{\rho_a \bar{U}_y \bar{d} a_{D1U}}{2m\omega^2} \tag{15}$$

$$\hat{a}_{D2U} = \frac{\rho_a \overline{d} a_{D2U}}{2m\omega^2} \tag{16}$$

$$\hat{a}_{D3U} = \frac{\rho_a \overline{d} a_{D2U}}{2m\omega^2 \overline{U}_{y}} \tag{17}$$

$$\hat{a}_{A0U} = \frac{\rho_a \overline{U}_z^2 \overline{d} a_{A0U}}{2m\omega^2} \tag{18}$$

$$\hat{a}_{A1U} = \frac{\rho_a \overline{U}_z \overline{d} a_{A1U}}{2m\omega^2} \tag{19}$$

$$\hat{a}_{A2U} = \frac{\rho_a \overline{d} a_{A2U}}{2m\omega^2} \tag{20}$$

$$\hat{a}_{A3U} = \frac{\rho_a \overline{d} a_{A3U}}{2m\omega^2 \overline{U}_z}$$
(21)

The aerodynamic coefficients of the y and z directions, and  $\overline{U}_{y,z}$  denotes the wind speed in the y and z directions. If  $\overline{U}_{y,z}$  is also in unsteady form, then

$$\hat{a}_{D0U} = \tilde{a}_{D0U} e^{2i\Omega t} \tag{22}$$

$$\hat{a}_{D1U} = \tilde{a}_{D1U} e^{i\Omega t} \tag{23}$$

$$\hat{a}_{A0U} = \tilde{a}_{A0U} e^{2i\Omega t} \tag{24}$$

$$\hat{a}_{AIU} = \tilde{a}_{AIU} e^{i\Omega t} \tag{25}$$

It is also noted that  $\hat{a}_{D0U}$  is zero for non-rotating body, and  $\hat{a}_{D1U}$  is  $2\pi$ .

This study investigates the internal resonance of a 3D nonlinear free-free beam. The main goal is focusing on the structural vibration analysis due to unsteady loads. The fluid-solid interaction aeroelastic problem is not considered in this

research. The boundary conditions for the free-free beam are simply the structural dynamic model. The dimensionless boundary conditions of the beam are:

$$\begin{cases} y''(0,t) = y'''(0,t) = y'''(1,t) = y'''(1,t) = 0\\ z''(0,t) = z'''(0,t) = z'''(1,t) = z'''(1,t) = 0\\ \gamma'(0,t) = \gamma'(1,t) = 0 \end{cases}$$
(26)

#### 2.2. Double section beam model

The equations of motion of a double section free-free beam (Fig. 2) can be re-written by using Eqs. (1)-(6) as follows:



Fig. 2 Schematic model of double section uniform free-free beam

$$m_{i}\ddot{v}_{(i)} + \overline{c}_{y}\dot{v}_{(i)} = -(D_{yy_{i}}v_{(i)}''' + D_{xx_{i}}(\gamma_{(i)}'w_{(i)}'') - D_{zz_{i}}[v_{(i)}'(v_{(i)}'v_{(i)}'' + w_{(i)}'w_{(i)}'')']' + (D_{yy_{i}} - D_{zz_{i}})[(w_{(i)}''\gamma_{(i)} - v_{(i)}''\gamma_{(i)}') - w_{(i)}''']\frac{\overline{x}_{i}}{\overline{t}_{i-1}}v_{(i)}''w_{(i)}'d\overline{x}_{i}]' - \frac{1}{2}\{v_{(i)}'\int_{\overline{t}_{i}}^{\overline{x}_{i}}m_{i}[\int_{\overline{t}_{i-1}}^{\overline{x}_{i}}(v_{(i)}'^{2} + w_{(i)}'^{2})d\overline{x}_{i}] \cdot d\overline{x}_{i}\}' + \overline{F}_{y}$$

$$(27)$$

$$m_{i}\ddot{w}_{(i)} + \overline{c}_{z}\dot{w}_{(i)} = -(D_{yy_{i}}w_{(i)}'')'' + D_{xx_{i}}(\gamma_{(i)}'v_{(i)}'') - D_{yy_{i}}[w_{(i)}'(v_{(i)}'v_{(i)}'' + w_{(i)}''w_{(i)}'')']' + (D_{yy_{i}} - D_{zz_{i}})[(v_{(i)}''\gamma_{(i)} + w_{(i)}''\gamma_{(i)}^{2})' + v_{(i)}'''\int_{\overline{l}_{i-1}}^{\overline{\lambda}_{i}}w_{(i)}'v_{(i)}'d\overline{x}_{i}]' - \frac{1}{2}\{w_{(i)}'\int_{\overline{l}_{i}}^{\overline{\lambda}_{i}}m_{i}[\int_{\overline{l}_{i-1}}^{\overline{\lambda}_{i}}(v_{(i)}'^{2} + w_{(i)}'^{2})d\overline{x}_{i}] \cdot d\overline{x}_{i}\}' + \overline{F}_{z}$$
(28)

$$\overline{j}_{x_{i}} \ddot{\gamma}_{(i)} + \overline{c}_{x} \dot{\gamma}_{(i)} = (D_{xx_{i}} \gamma'_{(i)})' + (D_{zz_{i}} - D_{yy_{i}})[(v_{(i)}'''^{2} - w_{(i)}''^{2})\gamma_{(i)} - v_{(i)}''w_{(i)}''] 
+ \overline{j}_{x_{i}}[(\int_{\overline{i}_{i-1}}^{\overline{x}_{i}} v_{(i)}''w_{(i)}d\overline{x}_{i})'' - (\dot{v}_{(i)}'w_{(i)}')'] 
+ (\overline{j}_{y_{i}} - \overline{j}_{z_{i}})[(\dot{v}_{(i)}'^{2} - \dot{w}_{(i)}'^{2})\gamma_{(i)} - \dot{w}_{(i)}'\dot{v}_{(i)}'] + \overline{M}_{x}$$
(29)

The index i = 1, 2 and represents the section number for this beam. For a circular cross-section beam, the moment of inertia  $j_{y_i} = j_{z_i}$ , and the side moment of inertia ratio equals 1 ( $\mu_{zy_i} = 1$ ), the structural dampings in the y- and z-dir are the same,  $c_y = c_z$ , the two sections of the beam are using the same material  $E_1 = E_2$ .

The wind force applied on the double-section beam is uniform and expressed as  $\bar{q}_{y,z}e^{i\bar{\Omega}_{y,z}\bar{t}}$  and is the same as in the case of uniform beam (Fig.1). This beam is subjected to unsteady aerodynamic forces in the y and z-dir. These forces are expressed as  $\bar{F}_{D,A}$ , where

$$q_{y,z} = \overline{q}_{y,z} / D_{yy1} \overline{l}$$

$$\Omega_{y,z} = \overline{\Omega}_{y,z} \overline{l}^2 \sqrt{D_{yy}} / m_1 \overline{l}^4$$
(30)

$$\begin{cases} \overline{F}_{D} = \frac{\rho_{a}\overline{U}_{y}^{2}\overline{d}_{1}}{2} \left( a_{D0U} + \frac{a_{D1U}}{\overline{U}_{y}} \frac{\partial \overline{y}_{(i)}(\overline{x},\overline{t})}{\partial \overline{t}} + \frac{a_{D2U}}{\overline{U}_{y}^{2}} \frac{\partial \overline{y}_{(i)}^{2}(\overline{x},\overline{t})}{\partial \overline{t}} + \frac{a_{D3U}}{\overline{U}_{y}^{3}} \frac{\partial \overline{y}_{(i)}^{3}(\overline{x},\overline{t})}{\partial \overline{t}} + \dots \right) \\ \overline{F}_{A} = \frac{\rho_{a}\overline{U}_{z}^{2}\overline{d}_{1}}{2} \left( a_{A0U} + \frac{a_{A1U}}{\overline{U}_{z}} \frac{\partial \overline{z}_{(i)}(\overline{x},\overline{t})}{\partial \overline{t}} + \frac{a_{A2U}}{\overline{U}_{z}^{2}} \frac{\partial \overline{z}_{(i)}^{2}(\overline{x},\overline{t})}{\partial \overline{t}} + \frac{a_{A3U}}{\overline{U}_{z}^{3}} \frac{\partial \overline{z}_{(i)}^{3}(\overline{x},\overline{t})}{\partial \overline{t}} + \dots \right) \end{cases}$$
(31)

The dimensionless forms are:

$$\begin{cases} F_D = \hat{a}_{D0U} + \hat{a}_{D1U} \dot{y} + \hat{a}_{D2U} \dot{y}^2 + \hat{a}_{D3U} \dot{y}^3 + \dots, \\ F_A = \hat{a}_{A0U} + \hat{a}_{A1U} \dot{z} + \hat{a}_{A2U} \dot{z}^2 + \hat{a}_{A3U} \dot{z}^3 + \dots, \end{cases}$$
(32)

where

$$\hat{a}_{D0U} = \frac{\rho_a \bar{U}_y^2 \bar{d}_1 a_{D0U}}{2m_1 \omega^2}$$
(33)

$$\hat{a}_{D1U} = \frac{\rho_a \overline{U}_y \overline{d}_1 a_{D1U}}{2m_1 \omega^2} \tag{34}$$

$$\hat{a}_{D2U} = \frac{\rho_a \bar{d}_1 a_{D2U}}{2m_1 \omega^2}$$
(35)

$$\hat{a}_{D3U} = \frac{\rho_a \bar{d}_1 a_{D3U}}{2m_1 \omega^2 \bar{U}_y}$$
(36)

$$\hat{a}_{A0U} = \frac{\rho_a \overline{U}_z^2 \overline{d}_1 a_{A0U}}{2m_1 \omega^2} \tag{37}$$

$$\hat{a}_{A1U} = \frac{\rho_a \overline{U}_z \overline{d}_1 a_{A1U}}{2m_1 \omega^2} \tag{38}$$

$$\hat{a}_{A2U} = \frac{\rho_a \overline{d}_1 a_{A2U}}{2m_i \omega^2} \tag{39}$$

$$\hat{a}_{A3U} = \frac{\rho_a \overline{d}_1 a_{A3U}}{2m_1 \omega^2 \overline{U}_z} \tag{40}$$

It is noted that Eqs. (30)-(32) are used for the two-stage beam. Instead of using "m" and "d" in Eqs. (11)-(21), Eqs. (30)-(32) use the dimensions of the first stage beam's mass " $m_1$ " and the characteristic length " $\bar{d}_1$ " to express the aerodynamic forces.

In order to make dimensionless beam equations, the following definitions are introduced.

$$\begin{cases} x_{i} = \overline{x}_{i} / \overline{l}, \ l_{i} = \overline{l}_{i} / \overline{l}, \ y_{i} = v_{i} / \overline{l}, \ z_{i} = w_{i} / \overline{l}, \ t = \overline{t} \sqrt{D_{yy_{1}} / m_{1} \overline{l}^{4}} \\ c_{y} = \overline{c}_{y} \overline{l}^{2} / \sqrt{m_{1} D_{yy_{1}}}, \ c_{z} = \overline{c}_{z} \overline{l}^{2} / \sqrt{m_{1} D_{yy_{1}}}, \ c_{x} = \overline{c}_{x} / \sqrt{m_{1} D_{yy_{1}}} j_{x_{1}} \\ j_{x_{i}} = \overline{j}_{x_{i}} / m_{1} \overline{l}^{2}, \ j_{y_{i}} = \overline{j}_{y_{i}} / m_{1} \overline{l}^{2}, \ j_{z_{i}} = \overline{j}_{z_{i}} / m_{1} \overline{l}^{2} \\ \mu_{xy_{i}} = D_{xx_{i}} / D_{yy_{i}}, \ \mu_{zy_{i}} = D_{zz_{i}} / D_{yy_{i}} \end{cases}$$

$$(41)$$

The dimensionless equations of motion of this double section beam can be obtained as:

$$\ddot{y}_{(i)} + y_{(i)}^{iv} = -(c_y - \hat{a}_{D1U})\dot{y}_{(i)} - \mu_{xy_i}(\gamma'_{(i)}z''_{(i)})' - [y'_{(i)}(y'_{(i)}y''_{(i)} + z'_{(i)}z''_{(i)})']' - \frac{1}{2}\{y'_{(i)}\int_{l_i}^{x_i}[\int_{l_{i-1}}^{x_i}(y'^2_{(i)} + z'^2_{(i)})dx_i] \cdot dx_i\}' + q_y e^{i\Omega_y t} + \hat{a}_{D0U}, \ (i = 1, 2)$$
(42)

$$\ddot{z}_{(i)} + z_{(i)}^{iv} = -(c_z - \hat{a}_{A0U})\dot{z}_{(i)} + \mu_{xy_i}(\gamma'_{(i)}y''_{(i)})' - [z'_{(i)}(y'_{(i)}y''_{(i)} + z'_{(i)}z''_{(i)})']' - \frac{1}{2} \{z'_{(i)}\int_{l_i}^{x_i} [\int_{l_{i-1}}^{x_i} (y'^2_{(i)} + z'^2_{(i)})dx_i] \cdot dx_i\}' + q_z e^{i\Omega_z t} + \hat{a}_{A0U}, \ (i = 1, 2)$$

$$\tag{43}$$

$$\ddot{\gamma}_{(i)} - \frac{\mu_{xy_i}}{j_{x_i}} \gamma_{(i)}'' = -c_x \dot{\gamma}_{(i)} + \left(\int_{l_{i-1}}^{x_i} y_{(i)}'' z_{(i)}' dx\right)'' - \left(\dot{y}_{(i)}' z_{(i)}'\right)' + M_x, \quad (i = 1, 2)$$

$$\tag{44}$$

The boundary conditions (Eq. (45)) and the compatibility equations (Eq. (46)) at the joint of the two beam sections are:

$$\begin{cases} y_{(1)}''(0,t) = y_{(1)}'''(0,t) = y_{(2)}''(1,t) = y_{(2)}'''(1,t) = 0\\ z_{(1)}''(0,t) = z_{(1)}'''(0,t) = z_{(2)}'''(1,t) = z_{(2)}'''(1,t) = 0\\ \gamma_{(1)}'(0,t) = \gamma_{(2)}'(1,t) = 0 \end{cases}$$
(45)

$$\begin{cases} y_{(1)}(\eta,t) = y_{(2)}(0,t), \ y'_{(1)}(\eta,t) = y'_{(2)}(0,t) \\ z_{(1)}(\eta,t) = z_{(2)}(0,t), \ z'_{(1)}(\eta,t) = z'_{(2)}(0,t) \\ \gamma_{(1)}(\eta,t) = \gamma_{(2)}(0,t) \end{cases}$$
(46)

where  $\eta = l_1 / (l_1 + l_2)$ .

### 2.3. Preturbation analysis

Perturbation methods allow the researchers to get good approximations for systems where the exact solutions are not all easy to be solved. This study adopted the method of multiple scales (MOMS) to analyze the frequency response and make the fixed point plots of this nonlinear system. The time scale was divided into fast and slow time scales. Suppose that  $T_0 = t$  is the fast-time term,  $T_1 = \varepsilon^2 t$  is the slow-time terms, and the expansions of each direction are:

$$\begin{cases} y_{(i)}(x,t;\varepsilon) = \varepsilon^{1} y_{(i)0}(x,T_{0},T_{1}) + \varepsilon^{3} y_{(i)1}(x,T_{0},T_{1}) + ..., \\ z_{(i)}(x,t;\varepsilon) = \varepsilon^{1} z_{(i)0}(x,T_{0},T_{1}) + \varepsilon^{3} z_{(i)1}(x,T_{0},T_{1}) + ..., \\ \gamma_{(i)}(x,t;\varepsilon) = \varepsilon^{1} \gamma_{(i)0}(x,T_{0},T_{1}) + \varepsilon^{3} \gamma_{(i)1}(x,T_{0},T_{1}) + ..., \end{cases}$$
(47)

where  $\varepsilon$  is the time scale of small disturbances and is a minimum value. For the sake of simplicity, the influence of high-order terms such as  $\varepsilon^5$ ,  $\varepsilon^7$ ... are neglected in the system. Two time scales ( $T_0$  and  $T_1$ ) are considered in this study. Under the assumption of nonlinear vibrations, the damping coefficient c is scaled as  $\varepsilon^2 c$ , the forcing term q is scaled as  $\varepsilon^3 q$ , respectively. To facilitate this analysis, only the first two terms of the unsteady aerodynamic force are extracted for the external force.

The orders of  $\hat{a}_{D0U}$  and  $\hat{a}_{D1U}$  in the  $F_D$  equation are chosen as  $\varepsilon^3$  and  $\varepsilon^2$ . Similarly, the orders of  $\hat{a}_{A0U}$  and  $\hat{a}_{A1U}$  in the  $F_A$  equation are set as  $\varepsilon^3$  and  $\varepsilon^2$ . These principles are substituted into Eqs. (42)-(44) and obtain the expansion of the equation of the  $\varepsilon^1$  order in the y-direction.

$$D_0^2 y_{(i)0} + y_{(i)0}^{i\nu} = 0, \ (i = 1, 2)$$
(48)

The equation of order  $\varepsilon^3$  is:

$$D_{0}^{2} y_{(i)1} + y_{(i)1}^{iv} = -2D_{0}D_{1}y_{(i)0} - (c_{y} - \hat{a}_{D1U})D_{0}y_{(i)0} - [y_{(i)0}'(y_{(i)0}'y_{(i)0}'' + z_{(i)0}'z_{(i)0}')']' - \frac{1}{2} \{y_{(i)0}'\int_{l_{i}}^{x_{i}} [\int_{l_{i-1}}^{x_{i}} (y_{(i)0}'^{2} + z_{(i)0}'^{2})dx_{i}] \cdot dx_{i}\}' + q_{y}e^{i\Omega_{y}t} + \hat{a}_{D0U}, \ (i = 1, 2)$$

$$(49)$$

The equation of the  $\varepsilon^1$  order in the *z* direction is:

$$D_0^2 z_{(i)0} + z_{(i)0}^{i\nu} = 0, \ (i = 1, 2)$$
(50)

The equation of order  $\varepsilon^3$  is:

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$$D_{0}^{2} z_{(i)1} + z_{(i)1}^{i\nu} = -2D_{0}D_{1} z_{(i)0} - (c_{z} - \hat{a}_{A1U})D_{0} z_{(i)0} - [z_{(i)0}'(y_{(i)0}'y_{(i)0}'' + z_{(i)0}'z_{(i)0}'')]' - \frac{1}{2} \{z_{(i)0}' \int_{l_{i}}^{x_{i}} [\int_{l_{i-1}}^{x_{i}} (y_{(i)0}'^{2} + z_{(i)0}'^{2})dx_{i}] \cdot dx_{i}\}' + q_{z}e^{i\Omega_{z}t} + \hat{a}_{A0U}, \quad (i = 1, 2)$$

$$(51)$$

The equation of the  $\varepsilon^1$  order in the  $\gamma$  direction is:

$$D_0^2 \gamma_{(i)0} - \frac{\mu_{xy_i}}{j_{x_i}} \gamma_{(i)0}'' = 0, \ (i = 1, 2)$$
(52)

The equation of order  $\varepsilon^3$  is:

$$D_0^2 \gamma_{(i)1} - \frac{\mu_{xy_i}}{j_{x_i}} \gamma_{(i)1}'' = -2D_0 D_1 \gamma_{(i)0} - c_x D_0 \gamma_{(i)0} + M_x, \quad (i = 1, 2)$$
(53)

The boundary conditions and the compatibility equations at the joint of the two beam sections are  $O(\varepsilon^1)$ :

$$\begin{cases} y_{(1)0}''(0,t) = y_{(1)0}'''(0,t) = y_{(2)0}''(1,t) = y_{(2)0}''(1,t) = 0, \\ z_{(1)0}''(0,t) = z_{(1)0}'''(0,t) = z_{(2)0}''(1,t) = z_{(2)0}'''(1,t) = 0, \\ \gamma_{(1)0}'(0,t) = \gamma_{(2)0}'(1,t) = 0. \end{cases}$$
(54)

$$\begin{cases} y_{(1)0}(\eta,t) = y_{(2)0}(0,t), \ y_{(1)0}'(\eta,t) = y_{(2)0}'(0,t), \\ z_{(1)0}(\eta,t) = z_{(2)0}(0,t), \ z_{(1)0}'(\eta,t) = z_{(2)0}'(0,t), \\ \gamma_{(1)0}(\eta,t) = \gamma_{(2)0}(0,t). \end{cases}$$
(55)

The equation of order  $\varepsilon^3$  is:

$$\begin{cases} y_{(1)1}''(0,t) = y_{(1)1}''(0,t) = y_{(2)1}''(1,t) = y_{(2)1}''(1,t) = 0, \\ z_{(1)1}''(0,t) = z_{(1)1}'''(0,t) = z_{(2)1}''(1,t) = z_{(2)1}'''(1,t) = 0, \\ \gamma_{(1)0}'(0,t) = \gamma_{(2)0}'(1,t) = 0. \end{cases}$$
(56)

$$\begin{aligned} y_{(1)1}(\eta,t) &= y_{(2)1}(0,t), \ y_{(1)1}'(\eta,t) = y_{(2)1}'(0,t), \\ z_{(1)1}(\eta,t) &= z_{(2)1}(0,t), \ z_{(1)1}'(\eta,t) = z_{(2)1}'(0,t), \\ \gamma_{(1)1}(\eta,t) &= \gamma_{(2)1}(0,t). \end{aligned}$$
(57)

## 2.4. 3D free-free beam free vibration analysis

The purpose of this section is to find the mode shapes of this vibration beam by the free vibration analysis. By using the MOMS (Section 2.3), the  $\varepsilon^1$  order in the *y* direction is expressed in Eq. (48).

The  $y_{(i)0}$  is divided into time and space by using the separation of variables, defined as  $y_{(i)0} = X(x)T(t)$ , and substituted in Eq. (48) to obtain:

$$\frac{X^{iv}(x)}{X(x)} = -\frac{D_0^2 T(t)}{T(t)} = \alpha^4$$
(58)

where  $\alpha$  is the eigenvalue of the system. The general solution of X(x) is assumed as:

$$X(x) = E_1(\sin\alpha x + \sinh\alpha x) + E_2(\sin\alpha x - \sinh\alpha x) + E_3(\cos\alpha x + \cosh\alpha x) + E_4(\cos\alpha x - \cosh\alpha x)$$
(59)

and substituted into the boundary conditions, which *reveals* that  $E_1 = E_3 = 0$  and that the characteristic equation is

$$\cos\alpha\cosh\alpha = 1 \tag{60}$$

then the mode shape of the  $i^{\text{th}}$  mode can be expressed as the following:

$$\varphi_{(i)}(x) = \sin \alpha_i x + \sinh \alpha_i(x) + \frac{\sin \alpha_i - \sinh \alpha_i}{\cosh \alpha_i - \cos \alpha_i} (\cos \alpha_i x + \cosh \alpha_i x)$$
(61)

## 3. Frequency Response

#### 3.1. Conditions for internal resonance

The natural frequency will be changed if the diameter and length of the beam are altered. The diameter ratios and the sectional length ratios of the compound beam are the main properties to trigger internal resonance. References [21 and 22] showed that the diameter ratio range is from 1/0.6 to 1/0.9 for the space rockets. The case of  $\bar{d}_1/\bar{d}_2 = 1/0.75$  is chosen as the test case.

The Transfer Matrix Method [17, 23] is used to find the relationship between modal frequency and the sectional length ratio ( $\eta$ ) for the beam. The conditions for internal resonance can be obtained by a certain combination of  $\eta$ . Taking an example of the *y*-direction motion and using the separation of variables, the following expression for  $y_{(i)0}$  can be obtained by:

$$y_{(i)0} = \varphi_{(i)}(x)\xi_{y_0}(t), \ (i = 1, 2)$$
(62)

Substituting Eq. (62) into Eq. (48) and dividing by  $\varphi_{(i)}(x)\xi_{y0}(t)$ , it yields:

$$\frac{\varphi_{(i)}^{iv}(x)}{\varphi_{(i)}(x)} = -\frac{D_0^2 \xi_{y_0}(t)}{\xi_{y_0}(t)} = \lambda_i^4, \ (i = 1, 2)$$
(63)

where  $\varphi_{(i)}(x)$  can be expressed as:

$$\varphi_{(1)}(x) = A_1 \cosh(\lambda_1 x) + B_1 \sinh(\lambda_1 x) + C_1 \cos(\lambda_1 x) + D_1 \sin(\lambda_1 x)$$
(64)

$$\varphi_{(2)}(x-\eta) = A_2 \cosh(\lambda_2(x-\eta)) + B_2 \sinh(\lambda_2(x-\eta)) + C_2 \cos(\lambda_2(x-\eta)) + D_2 \cos(\lambda_2(x-\eta)))$$
(65)

where  $\lambda_1$  is the eigen value. From [23],  $\lambda_1$  and  $\lambda_2$  satisfy the relationship  $\lambda_2 = (A_2 \bar{I}_1 / A_1 \bar{I}_2)^{1/4} \lambda_1$ , where A is the area of beam's cross-section and I is the moment of inertia. Since circular cross-sections are considered,  $\lambda_2 = (\bar{d}_1 / \bar{d}_2)^{1/2} \lambda_1 = 1.1547 \lambda_1$ .

The mode shape is expressed as:

$$\varphi(x) = \varphi_{(1)}(x) + \left[\varphi_{(2)}(x-\eta) - \varphi_{(1)}(x)\right] H \left(x-\eta\right)$$
(66)

where H() is the Heaviside function. The researchers define matrix  $\mathbf{P}^{(1)}$  and  $\mathbf{Q}^{(1)}$  as

$$\mathbf{P}^{(1)} = \begin{bmatrix} 1 & 0 & 1 & 0 \\ 0 & 1.1547\lambda_1 & 0 & 1.1547\lambda_1 \\ 0.4219\lambda_1^2 & 0 & -0.4219\lambda_1^2 & 0 \\ 0 & 0.4871\lambda_1^3 & 0 & -0.4871\lambda_1^3 \end{bmatrix}$$
(67)

$$\mathbf{Q}^{(1)} = \begin{bmatrix} \cosh(\eta\lambda_1) & \sinh(\eta\lambda_1) & \cos(\eta\lambda_1) & \sin(\eta\lambda_1) \\ \lambda_1 \sinh(\eta\lambda_1) & \lambda_1 \cosh(\eta\lambda_1) & -\lambda_1 \sin(\eta\lambda_1) & \lambda_1 \cos(\eta\lambda_1) \\ \lambda_1^2 \cosh(\eta\lambda_1) & \lambda_1^2 \sinh(\eta\lambda_1) & -\lambda_1^2 \cos(\eta\lambda_1) & -\lambda_1^2 \sin(\eta\lambda_1) \\ \lambda_1^3 \sinh(\eta\lambda_1) & \lambda_1^3 \cosh(\eta\lambda_1) & \lambda_1^3 \sin(\eta\lambda_1) & -\lambda_1^3 \cos(\eta\lambda_1) \end{bmatrix}$$
(68)

 $\mathbf{P}^{(1)}$  represents the moment of inertia of the 2<sup>nd</sup> section to the 1<sup>st</sup> section of the beam.  $\mathbf{Q}^{(1)}$  represents the displacement, slope, bending moment, and shear force shape function. The Transfer Matrix is expressed as:

$$\mathbf{T}^{(1)} = \left[\mathbf{P}^{(1)}\right]^{-1} \mathbf{Q}^{(1)}$$
(69)

and also satisfies the following condition,

$$\{ A_2 \quad B_2 \quad C_2 \quad D_2 \}^T = \mathbf{T}^{(1)} \{ A_1 \quad B_1 \quad C_1 \quad D_1 \}^T$$
(70)

From the boundary condition  $\varphi_{(1)}^{\prime\prime}(0) = \varphi_{(1)}^{\prime\prime\prime}(0)$ , it can be obtained by:

$$\begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 1 & 0 & -1 \end{bmatrix} \begin{cases} A_1 \\ B_1 \\ C_1 \\ D_1 \end{bmatrix} = \begin{cases} 0 \\ 0 \end{cases}$$
(71)

The external boundary conditions are:

$$\mathbf{B}_{\overline{I}} = \begin{bmatrix} \cosh S & \sinh S & -\cos S & -\sin S \\ \sinh S & \cosh S & \sin S & -\cos S \end{bmatrix}$$
(72)

where  $S = 1.1547(1 - \eta)\lambda_1$  and

$$\mathbf{B}_{\overline{i}} \left\{ A_2 \quad B_2 \quad C_2 \quad D_2 \right\}^T = \begin{cases} 0\\ 0 \end{cases}$$
(73)

Substituting Eq. (70) into Eq. (73), it yields:

$$\mathbf{H}\left\{A_{1} \quad B_{1} \quad C_{1} \quad D_{1}\right\}^{T} = \mathbf{B}_{\overline{I}}\mathbf{T}^{(1)}\left\{A_{1} \quad B_{1} \quad C_{1} \quad D_{1}\right\}^{T} = \begin{cases}0\\0\end{cases}$$
(74)

where  $\mathbf{H} = \mathbf{B}_{\bar{l}} \mathbf{T}^{(1)}$ 

$$\mathbf{H} = \begin{bmatrix} H_{11} & H_{12} & H_{13} & H_{14} \\ H_{21} & H_{22} & H_{23} & H_{24} \end{bmatrix}$$
(75)

and is the total transfer matrix. Eqs. (71) and (74) can be solved simultaneously,

$$\begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 1 & 0 & -1 \\ H_{11} & H_{12} & H_{13} & H_{14} \\ H_{21} & H_{22} & H_{23} & H_{24} \end{bmatrix} \begin{bmatrix} A_1 \\ B_1 \\ C_1 \\ D_1 \end{bmatrix} = \begin{cases} 0 \\ 0 \\ 0 \\ 0 \end{cases}$$
(76)

The characteristic equation for the eigen values is:

$$\det\left(\begin{bmatrix} 1 & 0 & -1 & 0\\ 0 & 1 & 0 & -1\\ H_{11} & H_{12} & H_{13} & H_{14}\\ H_{21} & H_{22} & H_{23} & H_{24} \end{bmatrix}\right) = 0$$
(77)

let

$$y_0(x,t) = \sum_{n=1}^{\infty} \xi_{y_0 n}(t) \varphi_n(x)$$
(78)

$$y_{1}(x,t) = \sum_{n=1}^{\infty} \xi_{y_{1}n}(t)\varphi_{n}(x)$$
(79)

Substituting into Eqs. (78) and (79) into Eqs. (48) and (49) and using orthogonal properties, the following dynamic equations can be obtained,  $O(\varepsilon^1)$ :

$$D_0^2 \xi_{y_0 m} + \lambda_{1m}^4 \xi_{y_0 m} = 0 \tag{80}$$

 $O(\varepsilon^3)$ :

The frequency as  $\omega_m = \lambda_{1m}^2$ , m=1, 2, 3... can be obtained from Eq. (80). The relationship between different frequency ratios and the beam sectional length ratios ( $\eta$ ) is shown in Fig. 3. The integer multiple of frequency ratios has the potential to trigger the internal resonance (I.R.). It is noted that higher orders of structural modes do not exist in linear cases. The energy transferring between lower and higher modes does not happen in the linear structural models. Since there are so many combinations of frequency ratios in this nonlinear system, the fixed point's plots are used to examine if the internal resonance really happens in these cases.

Besides, for the cases with many combinations of the ratios between higher and lower modes frequencies, the energy may not be possible to transfer between these modes. The possibility of higher modes frequency ratios to trigger I.R. is much less than the cases of small frequency ratios. This study also reveals that when  $\eta = 0.33 \& 0.51$ ,  $\omega_1: \omega_2 = 1:3$ ,  $\eta = 0.68 \& 0.86$ ,  $\omega_1: \omega_2 = 1:5$  and  $\eta = 0.25 \& 0.42$ ,  $\omega_1: \omega_2 = 1:6$  are possible to trigger I.R. According to Eq. (53), only cubic order of  $\xi$ exists in this system, which means only 1:3 I.R. exists in the 1<sup>st</sup> and 2<sup>nd</sup> modes. Therefore, only the case of 1:3 I.R. in the 1<sup>st</sup> and 2<sup>nd</sup> modes will be investigated in this study.

 $\eta = 0.33$  is chosen and substituted into Eqs. (68), (72). By using Eqs. (67)-(76), this study can obtain transfer matrix  $\mathbf{T}^{(1)}$  and find the first three eigenvalues: 4.03, 6.98, 10.04. The 1<sup>st</sup> section beam mode shape coefficients can be obtained by Eq. (76). The 2<sup>nd</sup> section beam mode shape coefficients can be obtained by Eq. (70). The mode shape functions of the 1<sup>st</sup> and 2<sup>nd</sup> beam sections can be obtained by Eqs. (65)-(66). Similarly, this study can gain other mode shapes for different  $\eta$ .



Fig. 3 Frequency ratios for different beam sectional length ratios

#### 3.2. Frequency analysis

Since 1:3 I.R. happens in the  $1^{st}$  and  $2^{nd}$  modes, the  $1^{st}$  and  $2^{nd}$  modes will be discussed in the following analysis. The solutions to the generalized coordinate are introduced:

$$\boldsymbol{\xi}_{0m} = \boldsymbol{B}_m(T_1)\boldsymbol{e}^{-i\boldsymbol{\zeta}_m} \boldsymbol{e}^{i\boldsymbol{\omega}_m T_0} + c\boldsymbol{c} \tag{82}$$

$$\dot{\xi}_{om} = D_0 \xi_{om} = i \omega_m B_m (T_1) e^{-i \zeta_m} e^{i \omega_m T_0} + cc$$
(83)

$$D_0 D_1 \xi_{_0m} = i \omega_m (B'_m(T_1) e^{-i\varsigma_m} e^{i\omega_m T_0} + cc) + \omega_m \zeta'_m (B_m(T_1) e^{-i\varsigma_m} e^{i\omega_m T_0} + cc)$$
(84)

where ()'  $\equiv D_1$ () and cc represents complex conjugate. In Eq. (81), the distributed load is assumed as  $q_m e^{i\Omega t} = q_m e^{i(\omega_m + \varepsilon^2 \sigma)T_0} = q_m e^{i\varepsilon^2 \sigma T_0} e^{i\omega_m T_0} = q_m e^{i\sigma T_1} e^{i\omega_m T_0}$ . If the solving procedure continues, terms containing the factors of the system frequencies (or harmonics) appear on the right-hand of Eq. (81). Terms such as these are called secular terms. Because of the secular terms, the solution of Eq. (81) increases without bound as t increases.

The time scale of  $\xi_1$  does not provide a small correction to time scale of  $\xi_0$ . The secular terms for  $\omega_1$ ,  $\omega_2 - 2\omega_1$  harmonics of the *y*-dir. 1<sup>st</sup> mode (m = 1) can be selected. Likewise, the secular terms due to harmonic numbers of  $\omega_2$ ,  $3\omega_1$  for the *y*-dir 2<sup>nd</sup> mode, due to harmonic numbers of  $\omega_1$ ,  $\omega_2 - 2\omega_1$  for the *z*-dir 1<sup>st</sup> mode, and due to harmonic numbers of  $\omega_2$ ,  $3\omega_1$  for the *z*-dir 2<sup>nd</sup> mode can be obtained, respectively. Refer to Appendix for the secular terms. Next is to let the secular terms as 0 to get the solvability conditions for solving the mode amplitudes in the frequency domain. The fixed point's plots can be obtained to analyze the frequency response for the 1<sup>st</sup> and 2<sup>nd</sup> modes of the *y*- and *z*-dir.



Fig. 4 Fixed point plot of diameter ratio = 1/0.75,  $\eta = 0.33$ , exciting the y-dir.,  $1^{st}$  mode



Fig. 5 Fixed point plot of diameter ratio = 1/0.75,  $\eta = 0.33$ , exciting the y-dir.,  $2^{nd}$  mode





Fig. 6 Fixed point plot of diameter ratio = 1/0.75,  $\eta = 0.51$ , exciting the y-dir.,  $1^{st}$  mode

Fig. 7 Fixed point plot of diameter ratio = 1/0.75,  $\eta = 0.51$ , exciting the y-dir.,  $2^{nd}$  mode

To verify the frequency domain results, the fourth order Runge-Kutta method (RK-4) is used to get the numerical results. The fourth order Runge–Kutta method is a family of implicit and explicit iterative methods in numerical analysis. It is used in temporal discretization for the approximate solutions of linear or nonlinear differential equations. In order to get the dynamic equations for the RK-4 method, the following expressions for y and z are chosen:

$$y = \sum_{n=1}^{\infty} \xi_{yn} \varphi_n \tag{85}$$

$$z = \sum_{n=1}^{\infty} \xi_{n} \varphi_n \tag{86}$$

Substituting Eqs. (85) and (86) into Eqs. (42) and (43) and by using the orthogonal property, the following dynamic equations can be obtained:

$$\begin{aligned} \ddot{\xi}_{y1} + (c_{y} - \hat{a}_{D1U})\dot{\xi}_{y1} + \omega_{1}^{2}\xi_{y1} + (\xi_{y1}^{3} + \xi_{y1}\xi_{z1}^{2})\tilde{C}_{1} + (\xi_{y1}^{2}\xi_{y2} + \xi_{y1}\xi_{z1}\xi_{z2})\tilde{C}_{4} + (\xi_{y1}^{2}\xi_{y2} + \xi_{y2}\xi_{z1}^{2})\tilde{C}_{5} \\ + (\xi_{y1}\xi_{y2}^{2} + \xi_{y1}\xi_{z2}^{2})\tilde{C}_{6} + (\xi_{y1}\xi_{y2}^{2} + \xi_{y2}\xi_{z1}\xi_{z2})\tilde{C}_{7} + (\xi_{y2}^{3} + \xi_{y2}\xi_{z2}^{2})\tilde{C}_{13} + [\xi_{y1}(\xi_{y1}\xi_{y1} + \xi_{y1}^{2} + \xi_{z1}\xi_{z1} + \xi_{z1}^{2})]\tilde{C}_{16} \\ + [\xi_{y1}(\xi_{y1}\xi_{y2} + 2\xi_{y1}\xi_{y2} + \xi_{y1}\xi_{y2} + \xi_{z1}\xi_{z2} + 2\xi_{z1}\xi_{z2} + \xi_{z1}\xi_{z2})]\tilde{C}_{17} + [\xi_{y2}(\xi_{y1}\xi_{y1} + \xi_{y1}^{2} + \xi_{z1}\xi_{z1} + \xi_{z1}^{2})]\tilde{C}_{15} \\ + [\xi_{y1}(\xi_{y2}\xi_{y2} + \xi_{y2}^{2} + \xi_{z2}\xi_{z2} + \xi_{z2}^{2})]\tilde{C}_{25} + [\xi_{y2}(\xi_{y1}\xi_{y2} + 2\xi_{y1}\xi_{y2} + \xi_{y1}\xi_{y2} + \xi_{z1}\xi_{z2} + 2\xi_{z1}\xi_{z2} + \xi_{z1}\xi_{z2})]\tilde{C}_{19} \\ + [\xi_{y2}(\xi_{y2}\xi_{y2} + \xi_{y2}^{2} + \xi_{z2}\xi_{z2} + \xi_{z2}^{2})]\tilde{C}_{25} \\ = \tilde{q}_{y1}e^{i\Omega_{y1}} + \tilde{G}_{1}\hat{a}_{D0U}\int_{0}^{1}\varphi_{1}dx \end{aligned}$$

$$(87)$$

$$\begin{aligned} \ddot{\xi}_{y2} + (c_{y} - \hat{a}_{D1U})\dot{\xi}_{y2} + \omega_{2}^{2}\xi_{y2} + (\xi_{y1}^{3} + \xi_{y1}\xi_{z1}^{2})\tilde{C}_{8} + (\xi_{y1}^{2}\xi_{y2} + \xi_{y1}\xi_{z1}\xi_{z2})\tilde{C}_{11} + (\xi_{y1}^{2}\xi_{y2} + \xi_{y2}\xi_{z1}^{2})\tilde{C}_{12} \\ + (\xi_{y1}\xi_{y2}^{2} + \xi_{y1}\xi_{z2}^{2})\tilde{C}_{14} + (\xi_{y1}\xi_{y2}^{2} + \xi_{y2}\xi_{z1}\xi_{z2})\tilde{C}_{15} + (\xi_{y2}^{3} + \xi_{y2}\xi_{z2}^{2})\tilde{C}_{10} + [\xi_{y1}(\xi_{y1}\xi_{y1} + \xi_{y1}^{2} + \xi_{z1}\xi_{z1} + \xi_{z1}^{2})]\tilde{C}_{21} \\ + [\xi_{y1}(\xi_{y1}\xi_{y2} + 2\xi_{y1}\xi_{y2} + \xi_{y1}\xi_{y2} + \xi_{z1}\xi_{z2} + 2\xi_{z1}\xi_{z2} + \xi_{z1}\xi_{z2})]\tilde{C}_{22} + [\xi_{y2}(\xi_{y1}\xi_{y1} + \xi_{y1}^{2} + \xi_{z1}\xi_{z1} + \xi_{z1}^{2})]\tilde{C}_{23} \\ + [\xi_{y1}(\xi_{y2}\xi_{y2} + \xi_{y2}^{2} + \xi_{z2}\xi_{z2} + \xi_{z2}^{2})]\tilde{C}_{24} + [\xi_{y2}(\xi_{y1}\xi_{y2} + 2\xi_{y1}\xi_{y2} + \xi_{y1}\xi_{y2} + \xi_{z1}\xi_{z2} + 2\xi_{z1}\xi_{z2} + \xi_{z1}\xi_{z2})]\tilde{C}_{26} \\ + [\xi_{y2}(\xi_{y2}\xi_{y2} + \xi_{y2}^{2} + \xi_{z2}\xi_{z2} + \xi_{z2}^{2})]\tilde{C}_{24} \\ = \tilde{q}_{y2}e^{i\Omega_{y}t} + \tilde{G}_{2}\hat{a}_{D0U}\int_{0}^{1}\varphi_{2}dx \end{aligned}$$
(88)

$$\begin{aligned} \ddot{\xi}_{z1}^{z} + (c_{z} - \hat{a}_{AU})\dot{\xi}_{z1}^{z} + a_{1}^{2}\xi_{z1}^{z} + (\xi_{z1}^{3} + \xi_{z1}\xi_{y1}^{2})\tilde{C}_{1} + (\xi_{z1}^{2}\xi_{z2}^{z} + \xi_{z1}\xi_{y1}\xi_{y2})\tilde{C}_{4} + (\xi_{z1}^{2}\xi_{z2}^{z} + \xi_{z2}\xi_{y1}^{2})\tilde{C}_{5} \\ + (\xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{y2}^{2})\tilde{C}_{6}^{z} + (\xi_{z1}\xi_{z2}^{z} + \xi_{z2}\xi_{y1}\xi_{y2})\tilde{C}_{7} + (\xi_{z1}^{3}\xi_{z2}^{z} + \xi_{z1}\xi_{y2}^{2})\tilde{C}_{13}^{z} + [\xi_{z1}(\xi_{z1}\xi_{z1} + \xi_{z1}^{2} + \xi_{y1}\xi_{y1} + \xi_{y1}^{2})]\tilde{C}_{16} \\ + [\xi_{z1}(\xi_{z1}\xi_{z2}^{z} + 2\xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{y1}\xi_{y2} + 2\xi_{y1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z})]\tilde{C}_{17}^{z} + \tilde{G}_{1}[\xi_{z2}(\xi_{z1}\xi_{z1} + \xi_{z1}^{z} + \xi_{y1}\xi_{y1} + \xi_{y1}^{2})]\tilde{C}_{15} \\ + [\xi_{z1}(\xi_{z2}\xi_{z2}^{z} + \xi_{z2}^{z} + \xi_{y2}\xi_{y2}^{z} + \xi_{y2}^{z})]\tilde{C}_{25}^{z} + [\xi_{z2}(\xi_{z1}\xi_{z2}^{z} + 2\xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z2}\xi_{z1}^{z})\tilde{C}_{11} \\ + [\xi_{z1}(\xi_{z2}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z2}\xi_{z2}^{z} + \xi_{z2}^{z}\xi_{z1}^{z})\tilde{C}_{12} \\ + (\xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z1}\xi_{z2}^{z} + \xi_{z1}^{z} + \xi_{z2}^{z} + \xi$$

where the  $\tilde{C}_s$  are the mode shape integrations and are shown in the Appendix. Eqs. (87)-(90) can be solved simultaneously by using RK-4.



Fig. 8 Phase plot of diameter ratio = 1/0.75,  $\eta = 0.33$ , exciting the y-dir., 1<sup>st</sup> mode



Fig. 9 Phase plot of diameter ratio = 1/0.75,  $\eta = 0.51$ , exciting the y-dir., 1<sup>st</sup> mode

The phase plots are shown in Figs. 8-11. It is recorded that Figs. 8-9 are the cases for the 1<sup>st</sup> mode excited (the excitation frequency is the 1<sup>st</sup> mode's natural frequency). Figs. 10-11 are the cases for the 2<sup>nd</sup> mode excited (the excitation frequency is the 1<sup>st</sup> mode's natural frequency). The Displ. and Vel. represent the  $\xi_{y1,y2,z1}$  and  $\dot{\xi}_{y1,y2,z1}$ , respectively. The converged displacements in time domain in Figs. 8-11 agree with the amplitudes form fixed point plots and show the I.R. happens in these cases.



It is noted that the frequency analysis of the uniform beam can also be done by using Eqs. (8)-(10) and apply MOMS, select the secular terms for different harmonics of different modes and solve the solvability conditions to make the fixed point plots, since the procedure is similar to the case of double-section beam, and will not detail here.



Fig. 11 Phase plot of diameter ratio = 1/0.75,  $\eta = 0.51$ , y-dir., exciting the 2<sup>nd</sup> mode

## 4. Wind Pressure Effects

#### 4.1. Diameter ratio and length ratio

Section 3.2 showed that when the diameter ratio  $\bar{d_1}/\bar{d_2} = 1/0.75$  and the length ratio  $\eta = 0.33\&0.51$ , the 1:3 I.R. internal resonance occurs in the 1<sup>st</sup> and 2<sup>nd</sup> modes. Various diameter ratios from 1/0.6 to 1/0.9 will be discussed in this Section. Fig. 12 shows that when the diameter ratios are less than 1/0.78, there will be no I.R. When the diameter ratios are greater than 1/0.78, there will be two possible combinations for the length ratios to trigger I.R. Since there is no integer multiple of the frequency ratio for the first 3 modes in the uniform beam, the I.R. will not occur. However, the primary resonance will be examined for the uniform beam in the following Section.

#### 4.2. Wind pressure and the beam amplitude

Section 4.1 finds that the I.R. could be triggered in some certain combinations of diameter ratios and beam sectional length ratios. The I.R. cannot happen in the uniform beam, because there is no integer multiple of frequency ratios. Next step is to study the effect of the dimensionless wind pressure  $\tilde{q}_m$  (Refer to Appendix for the definition) on this beam amplitude. For the case of a uniform beam, the value of the dimensionless wind pressure  $\tilde{q}_{y1}$  varies from 1-10.

The responding amplitudes in both y- and z-D.O.F. can be found from Eqs. (8)-(10) by using MOMS method. The aerodynamic force is treated as the external load in this study. Instead of Reynolds number, the dimensionless wind pressure  $\tilde{q}_{y1}$  is considered in Figs. 13-17.

Fig. 13 demonstrates the primary resonance phenomenon in an isotropic uniform beam when the 1<sup>st</sup> mode's natural frequency in the *y*-dir. is excited. Fig. 13 presents that the amplitudes in the *y*-dir. increases linearly when  $\tilde{q}_{y1}$  increases when  $\tilde{q}_{y1} \leq 7$ . The amplitudes in the *z*-dir. seem to be unexcited. However, in the cases of  $\tilde{q}_{y1} > 7$ , the amplitudes in the *z*-dir. increase notably.

This implies that the energy is transferring from one D.O.F. to another D.O.F. under these conditions. When  $\tilde{q}_{y1} = 10$ , the amplitude in the *z*-dir. is larger than the *y*-dir. This is a typical primary resonance. The result shows the primary resonance can happen under such conditions.



Fig. 12 Frequency ratio plot of a double section beam, exciting the y-dir. 1st mode



Fig. 13 Beam amplitude-wind pressure plot of the uniform isotropic beam, exciting the *y*-dir. 1<sup>st</sup> mode's natural frequency

Fig. 14 indicates the beam amplitude-wind pressure plot of the double section beam  $\bar{d}_1/\bar{d}_2 = 1/0.75$ ,  $\eta = 0.33$ , when the y-dir. 1<sup>st</sup> mode is excited. Fig. 15 reveals the beam amplitude-wind pressure plot of the double section beam  $\bar{d}_1/\bar{d}_2 = 1/0.75$ ,  $\eta = 0.51$ , when the y-dir. 1<sup>st</sup> mode is excited. The amplitude of  $B_{y1}$  is always larger than the other cases and increases linearly as the wind pressure increases. The case of  $\eta = 0.33$  represents a more flexible beam than the case of  $\eta = 0.51$ . This is the reason that  $B_{y1}$  has the largest amplitude than the other cases.



Fig. 14 Beam amplitude-wind pressure plot of the double section beam  $\bar{d}_1/\bar{d}_2 = 1/0.75$ ,  $\eta = 0.33$ , exciting the y-dir. 1<sup>st</sup> mode



Fig. 15 Beam amplitude-wind pressure plot of the double section beam  $\bar{d}_1/\bar{d}_2 = 1/0.75$ ,  $\eta = 0.51$ , exciting the *y*-dir. 1<sup>st</sup> mode

Figs. 16-17 are the beam amplitude-wind pressure plots of the double section beam  $\bar{d}_1/\bar{d}_2 = 1/0.75$  when the y-dir. 2<sup>nd</sup> mode is excited and when  $\eta = 0.33$  and  $\eta = 0.51$ , respectively. Even if the 2nd mode is excited and  $B_{y2}$  increases as the wind pressure increases, the unexcited D.O.F. amplitude  $B_{y1}$  is still larger than that of the excited D.O.F. This implies the energy is transferring in between these two modes.

The internal resonance is triggered. In comparing with the primary resonance, the 3:1 I.R. happens earlier (less wind pressure) than the primary resonance (evidenced by the growth of  $B_{z1}$ ). The internal resonance has greater effects than the primary resonance and should be noticed under this forcing condition.



Fig. 16 Beam amplitude-wind pressure plot of the double section beam  $\bar{d}_1/\bar{d}_2 = 1/0.75$ ,  $\eta = 0.33$ , exciting the *y*-dir. 2<sup>nd</sup> mode



Fig. 17 Beam amplitude-wind pressure plot of the double section beam  $\bar{d}_1/\bar{d}_2 = 1/0.75$ ,  $\eta = 0.51$ , exciting the *y*-dir. 2<sup>nd</sup> mode

## 5. Conclusions

This study investigates the internal resonance of a double-section beam with cubic nonlinearities. This model can be applied in a wide range of engineering problems, such as rocket and missile structures. Different ratios of beam section lengths and diameters in a variety of external loads (aerodynamic forces or flight speeds of the rockets or missiles) are studied. A primary resonance occurs on a uniform free-free beam at certain flight speed. The three-to-one internal resonance is also triggered within the 1<sup>st</sup> and the 2<sup>nd</sup> modes in certain combinations of beam length and diameter ratios. The phase plots and the time marching numerical method are used to verify the semi-analytical results. The findings are concluded as follows:

- (1) For a uniform beam, and when wind pressure  $\bar{q}_{y1} \ge 10$ , primary resonance occurs.
- (2) The internal resonance does not happen in the uniform beam case, because the frequency ratios of different modes in the uniform beam are not equal to integer multiples.
- (3) For the double sectional beam and the diameter ratio  $\bar{d}_1/\bar{d}_2 = 1/0.75$ , the length ratio  $\eta = 0.33 \pm 0.51$ , the 1:3 internal resonance in the 1<sup>st</sup> and 2<sup>nd</sup> modes occurs.
- (4) When the diameter ratios are less than 1/0.78, there will be no I.R. While the diameter ratios are greater than 1/0.78, there will be two possible combinations for the length ratios to trigger I.R.
- (5) For the case of the double section beam, the internal resonance has greater effects than the primary resonance and should be noticed in this forcing condition.

This study reveals the fight conditions to trigger primary resonance or internal resonance. However, the vibration of the beam structure still exists. The investigations of vibration reduction on the 3D beam or rocket model deserve an extension study on this research. The tuned mass damper (TMD) or the damping ring may be added to see the damping effects on the vibration beam. Since the nonlinear phenomenon is difficult to be observed experimentally, the experimental setup for verification of the present predictions is suggested in future research.

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## **Conflicts of Interest**

The authors declare no conflict of interest

## Appendix

The secular terms for  $\omega_1$ ,  $\omega_2 - 2\omega_1$  harmonics of the y-dir. 1<sup>st</sup> mode (m = 1) are:

$$-2(i\omega_{l}B'_{yl}e^{-i\varsigma_{y1}} + \omega_{l}\varsigma'_{y1}B_{y1}e^{-i\varsigma_{y1}}) - (c_{y} - \hat{a}_{DU})i\omega_{l}B_{y1}e^{-i\varsigma_{y1}} - 3B_{y1}^{2}\overline{B}_{y1}\overline{B}_{y1}e^{-i\varsigma_{y1}}\tilde{C}_{1} - \overline{B}_{y1}^{2}B_{y2}e^{-i(-2\varsigma_{y1}+\varsigma_{y2})}\tilde{C}_{2}$$

$$-2B_{y1}B_{y2}\overline{B}_{y2}e^{-i\varsigma_{y1}}\tilde{C}_{3} - (2B_{y1}B_{z1}\overline{B}_{z1}e^{-i\varsigma_{y1}} + \overline{B}_{y1}B_{z1}^{2}e^{-(-\varsigma_{y1}+2\varsigma_{z1})})\tilde{C}_{1} - \overline{B}_{y1}\overline{B}_{z1}B_{z2}e^{-(-\varsigma_{y1}-\varsigma_{z1}+\varsigma_{z2})}\tilde{C}_{4}$$

$$-B_{y2}\overline{B}_{z1}^{2}e^{-i\varsigma_{y1}}\tilde{C}_{5} - 2B_{y1}B_{z2}\overline{B}_{z2}e^{-i\varsigma_{y1}}\tilde{C}_{6} - (B_{y2}B_{z1}\overline{B}_{z2}e^{-i(\varsigma_{y2}+\varsigma_{z1}-\varsigma_{z2})} + \overline{B}_{y2}B_{z1}B_{z2}e^{-i(--\varsigma_{y2}+\varsigma_{z1}+\varsigma_{z2})})\tilde{C}_{7}$$

$$+\omega_{1}^{2}[3B_{y1}^{2}\overline{B}_{y1}e^{-i\varsigma_{y1}}\tilde{C}_{16} + \overline{B}_{y1}^{2}B_{y2}e^{-i(-2\varsigma_{y1}+\varsigma_{y2})}(\tilde{C}_{17} + \tilde{C}_{18}) + 2B_{y1}B_{y2}\overline{B}_{y2}e^{-i\varsigma_{y1}}\tilde{C}_{19}] + \omega_{2}^{2}[\overline{B}_{y1}^{2}B_{y2}e^{-i(-2\varsigma_{y1}+\varsigma_{y2})}\tilde{C}_{17}$$

$$+2B_{y1}B_{y2}\overline{B}_{y2}e^{-i\varsigma_{y1}}(\tilde{C}_{20} + \tilde{C}_{19})] - \omega_{1}^{2}B_{y1}^{2}\overline{B}_{y1}e^{-i\varsigma_{y1}}\tilde{C}_{16} - 2\omega_{l}\omega_{2}\overline{B}_{y1}^{2}B_{y2}e^{-i(-2\varsigma_{y1}+\varsigma_{y2})}\tilde{C}_{17} + \omega_{1}^{2}\overline{B}_{y2}^{2}B_{y2}e^{-i(-2\varsigma_{y1}+\varsigma_{y2})}\tilde{C}_{18}$$

$$-2\omega_{2}^{2}B_{y1}B_{y2}\overline{B}_{y2}e^{-i\varsigma_{y1}}\tilde{C}_{20} + \omega_{1}^{2}[(2B_{y1}B_{z1}\overline{B}_{z1}e^{-i\varsigma_{y1}} + \overline{B}_{y1}B_{z1}e^{-(-\varsigma_{y1}+2\varsigma_{z1})})\tilde{C}_{16} + \overline{B}_{y1}\overline{B}_{z1}B_{z2}e^{-(-\varsigma_{y1}-\varsigma_{z1}+\varsigma_{z2})}\tilde{C}_{17}$$

$$+B_{y2}\overline{B}_{z1}^{2}e^{-(\varsigma_{y2}-2\varsigma_{z1})}\tilde{C}_{18} + (B_{y2}B_{z1}\overline{B}_{z2}e^{-i(\varsigma_{y2}+\varsigma_{z1}-\varsigma_{z2})} + \overline{B}_{y2}B_{z1}B_{z2}e^{-i(-\varsigma_{y2}+\varsigma_{z1}+\varsigma_{z2})}\tilde{C}_{19}] + \omega_{2}^{2}[\overline{B}_{y1}\overline{B}_{z1}B_{z2}e^{-(-\varsigma_{y1}-\varsigma_{z1}+\varsigma_{z2})}\tilde{C}_{17}$$

$$+B_{y2}\overline{B}_{z1}^{2}e^{-(\varsigma_{y1}-2\varsigma_{z1})}\tilde{C}_{18} + (B_{y2}B_{z1}\overline{B}_{z2}e^{-i(\varsigma_{y2}+\varsigma_{z1}-\varsigma_{z2})} + \overline{B}_{y2}B_{z1}B_{z2}e^{-i(-\varsigma_{y2}+\varsigma_{z1}+\varsigma_{z2})}\tilde{C}_{19}] + \omega_{1}^{2}(-2B_{y1}B_{z1}\overline{B}_{z1}e^{-(-\varsigma_{y1}-\varsigma_{z1}+\varsigma_{z2})}\tilde{C}_{17}$$

$$+B_{y2}B_{z1}^{2}e^{-(-\varsigma_{y1}-2\varsigma_{z1})}\tilde{C}_{18} + (B_{y2}B_{z1}\overline{B}_{z2}e^{-i(\varsigma_{y2}+\varsigma_{z1}-\varsigma_{z2})} + \overline{B}_{y2}B_{z1}B_{z2}e^{-i(-\varsigma_{y2}+\varsigma_{z1}+\varsigma_{z2})}\tilde{C}_{17} + \omega_{1}^{2}B_{y2}\overline{B}_{z1}^{2}e^{-(-\varsigma_{y1}-\varsigma_{z1}+\varsigma_{z2})}\tilde{C}_{17}$$

$$+B_{y2}B_{z$$

The secular terms for  $\omega_2$ ,  $3\omega_1$  harmonics of the y-dir.  $2^{nd}$  mode are:

$$\begin{aligned} -2(i\omega_{2}B'_{y2}e^{-i\varsigma_{y2}} + \omega_{2}\varsigma'_{y2}B_{y2}e^{-i\varsigma_{y2}}) - (c_{y} - \hat{a}_{D1U})i\omega_{2}B_{y2}e^{-i\varsigma_{y2}} - B_{y1}^{3}e^{-3i\varsigma_{y1}}\tilde{C}_{8} - 2B_{y1}\bar{B}_{y1}B_{y2}e^{-i\varsigma_{y2}}\tilde{C}_{9} - 3B_{y2}^{2}\bar{B}_{y2}e^{-i\varsigma_{y2}}\tilde{C}_{10} \\ -B_{y1}B_{z1}^{2}e^{-i(\varsigma_{y1}+2\varsigma_{z1})}\tilde{C}_{8} - (B_{y1}\bar{B}_{z1}B_{z2}e^{-i(\varsigma_{y1}-\varsigma_{z1}+\varsigma_{z2})} + \bar{B}_{y1}B_{z1}B_{z2}e^{-i(-\varsigma_{y1}+\varsigma_{z1}+\varsigma_{z2})})\tilde{C}_{11} - 2B_{y2}B_{z1}\bar{B}_{z1}e^{-i\varsigma_{y2}}\tilde{C}_{12} \\ -(2B_{y2}B_{z2}\bar{B}_{z2}e^{-i\varsigma_{y2}} + \bar{B}_{y2}B_{z2}^{2}e^{-i(-\varsigma_{y2}+2\varsigma_{z2})})\tilde{C}_{10} + \omega_{1}^{2}[B_{y1}^{3}e^{-3i\varsigma_{y1}}\tilde{C}_{21} + 2B_{y1}\bar{B}_{y1}B_{y2}e^{-i\varsigma_{y2}}(\tilde{C}_{22} + \tilde{C}_{23})] \\ +\omega_{2}^{2}[2B_{y1}\bar{B}_{y1}B_{y2}e^{-i\varsigma_{y2}}\tilde{C}_{22} + 3B_{y2}^{2}\bar{B}_{y2}e^{-i\varsigma_{y2}}\tilde{C}_{24}] + \tilde{G}_{2}\omega_{1}^{2}B_{y1}^{3}e^{-3i\varsigma_{y1}}\tilde{C}_{21} - 2\omega_{1}^{2}B_{y1}\bar{B}_{y1}B_{y2}e^{-i\varsigma_{y2}}\tilde{C}_{23} - \omega_{1}^{2}B_{y2}\bar{B}_{y2}e^{-i\varsigma_{y2}}\tilde{C}_{24} \\ +\omega_{1}^{2}[B_{y1}B_{z1}e^{-i(\varsigma_{y1}+2\varsigma_{z1})}\tilde{C}_{21} + (B_{y1}\bar{B}_{z1}B_{z2}e^{-i(\varsigma_{y1}-\varsigma_{z1}+\varsigma_{z2})} + \bar{B}_{y1}B_{z1}B_{z2}e^{-i(-\varsigma_{y1}+\varsigma_{z1}+\varsigma_{z2})})\tilde{C}_{22} + 2B_{y2}B_{z1}\bar{B}_{z1}e^{-i\varsigma_{y2}}\tilde{C}_{23}] \\ -2\omega_{1}^{2}B_{y2}B_{z1}\bar{B}_{z1}e^{-i\varsigma_{y2}}\tilde{C}_{23} + \omega_{2}^{2}[(B_{y1}\bar{B}_{z1}B_{z2}e^{-i(\varsigma_{y1}-\varsigma_{z1}+\varsigma_{z2})} + \bar{B}_{y1}B_{z1}B_{z2}e^{-i(-\varsigma_{y1}+\varsigma_{z1}+\varsigma_{z2})})\tilde{C}_{22} \\ +(2B_{y2}B_{z2}\bar{B}_{z2}e^{-i(-\varsigma_{y2}+2\varsigma_{z2})}\tilde{C}_{23} + \omega_{2}^{2}[(B_{y1}\bar{B}_{z1}B_{z2}e^{-i(-\varsigma_{y1}-\varsigma_{z1}+\varsigma_{z2})} + \bar{B}_{y1}B_{z1}B_{z2}e^{-i(-\varsigma_{y1}+\varsigma_{z1}+\varsigma_{z2})})\tilde{C}_{22} \\ +(2B_{y2}B_{z2}\bar{B}_{z2}e^{-i(-\varsigma_{y2}+2\varsigma_{z2})}\tilde{C}_{23} + \omega_{2}^{2}[(B_{y1}\bar{B}_{z1}B_{z2}e^{-i(-\varsigma_{y1}+\varsigma_{z1}+\varsigma_{z2})} + \bar{B}_{y1}B_{z1}B_{z2}e^{-i(-\varsigma_{y1}+\varsigma_{z1}+\varsigma_{z2})})\tilde{C}_{22} \\ +\omega_{2}^{2}(-2B_{y2}B_{z2}\bar{B}_{z2}e^{-i(-\varsigma_{y1}-\varsigma_{z1}+\varsigma_{z2})} + \bar{B}_{y1}B_{z1}B_{z2}e^{-i(-\varsigma_{y1}+\varsigma_{z1}+\varsigma_{z2})})\tilde{C}_{24} \\ +\omega_{2}^{2}(-2B_{y2}B_{z2}\bar{B}_{z2}e^{-i(\varsigma_{y2}}+\bar{B}_{y2}B_{z2}^{2}e^{-i(-\varsigma_{y2}+2\varsigma_{z2})})\tilde{C}_{24} + \tilde{Q}_{y2}e^{i\sigma\tau_{1}} \\ +\omega_{2}^{2}(-2B_{y2}B_{z2}\bar{B}_{z2}e^{-i(-\varsigma_{y2}+2\varsigma_{z2})})\tilde{C}_{24} + \tilde{Q}_{y2}e^{i\sigma\tau_{1}} \\ +\omega_{2}^{2}(-2B_{$$

The secular terms for  $\omega_1$ ,  $\omega_2 - 2\omega_1$  harmonics of the *z*-dir and 1<sup>st</sup> mode are:

$$\begin{split} -2(i\omega_{l}B'_{z1}e^{-i\zeta_{z1}} + \omega_{l}\zeta'_{z1}B_{z1}e^{-i\zeta_{z1}}) - (c_{z} - \hat{a}_{AlU})i\omega_{l}B_{z1}e^{-i\zeta_{z1}} - 3B^{2}_{z1}\overline{B}_{z1}e^{-i\zeta_{z1}}\tilde{C}_{1} - \overline{B}^{2}_{z1}B_{z2}e^{-i(-2\zeta_{z1}+\zeta_{z2})}\tilde{C}_{2} - 2B_{z1}B_{z2}\overline{B}_{z2}e^{-i\zeta_{z1}}\tilde{C}_{3} \\ -(2B_{z1}B_{y1}\overline{B}_{y1}e^{-i\zeta_{z1}} + \overline{B}_{z1}B^{2}_{y1}e^{-(-\zeta_{z1}+2\zeta_{y1})})\tilde{C}_{1} - \overline{B}_{z1}\overline{B}_{y1}B_{y2}e^{-(-\zeta_{z1}-\zeta_{y1}+\zeta_{y2})}\tilde{C}_{4} - B_{z2}\overline{B}^{2}_{y1}e^{-(\zeta_{z2}-2\zeta_{y1})}\tilde{C}_{5} - 2B_{z1}B_{y2}\overline{B}_{y2}e^{-i\zeta_{z1}}\tilde{C}_{6} \\ -(B_{z2}B_{y1}\overline{B}_{y2}e^{-i(\zeta_{z2}+\zeta_{y1}-\zeta_{y2})} + \overline{B}_{z2}B_{y1}B_{y2}e^{-i(-\zeta_{z2}+\zeta_{y1}+\zeta_{y2})})\tilde{C}_{7} + \omega_{1}^{2}[3B^{2}_{z1}\overline{B}_{z1}e^{-i\zeta_{z1}}\tilde{C}_{16} + \overline{B}^{2}_{z1}B_{z2}e^{-i(-2\zeta_{z1}+\zeta_{z2})}(\tilde{C}_{17} + \tilde{C}_{18}) \\ +2B_{z1}B_{z2}\overline{B}_{z2}e^{-i\zeta_{z1}}\tilde{C}_{19}] + \omega_{2}^{2}[\overline{B}^{2}_{z1}B_{z2}e^{-i(-2\zeta_{z1}+\zeta_{z2})}\tilde{C}_{17} + 2B_{z1}B_{z2}\overline{B}_{z2}e^{-i\zeta_{z1}}(\tilde{C}_{20} + \tilde{C}_{19})] - \omega_{1}^{2}B^{2}_{z1}\overline{B}_{z1}e^{-i\zeta_{z1}}\tilde{C}_{16} \\ -2\omega_{1}\omega_{2}\overline{B}^{2}_{z1}B_{z2}e^{-i(-2\zeta_{z1}+\zeta_{z2})}\tilde{C}_{17} + \omega_{1}^{2}\overline{B}^{2}_{z1}B_{z2}e^{-i(-2\zeta_{z1}+\zeta_{z1}+\zeta_{z2})}\tilde{C}_{18} - 2\omega_{2}^{2}B_{z1}B_{z2}\overline{B}_{z2}e^{-i\zeta_{z1}}\tilde{C}_{20} \\ +\omega_{1}^{2}[(2B_{z1}B_{y1}\overline{B}_{y1}e^{-i\zeta_{z1}} + \overline{B}_{z1}B^{2}_{y1}e^{-(-\zeta_{z1}+2\zeta_{y1})})\tilde{C}_{16} + \overline{B}_{z1}\overline{B}_{y1}B_{y2}e^{-(-\zeta_{z1}-\zeta_{y1}+\zeta_{y2})}\tilde{C}_{17} + B_{z2}\overline{B}^{2}_{y1}e^{-(\zeta_{z2}-2\zeta_{y1})}\tilde{C}_{18} \\ +(B_{z2}B_{y1}\overline{B}_{y2}e^{-i(\zeta_{z2}+\zeta_{y1}-\zeta_{y2})} + \overline{B}_{z2}B_{y1}B_{y2}e^{-i(-\zeta_{z2}+\zeta_{y1}+\zeta_{y2})})\tilde{C}_{19}] + \omega_{2}^{2}[\overline{B}_{z1}\overline{B}_{y1}B_{y2}e^{-(-\zeta_{z1}-\zeta_{y1}+\zeta_{y2})}\tilde{C}_{17} + 2B_{z1}B_{y2}B^{2}_{y2}e^{-i\zeta_{z1}}\tilde{C}_{20} \\ +(B_{z2}B_{y1}\overline{B}_{y2}e^{-i(\zeta_{z2}+\zeta_{y1}-\zeta_{y2})} + \overline{B}_{z2}B_{y1}B_{y2}e^{-i(-\zeta_{z2}+\zeta_{y1}+\zeta_{y2})})\tilde{C}_{19}] + \omega_{1}^{2}(-2B_{z1}B_{y1}\overline{B}_{y1}e^{-i\zeta_{z1}} + \overline{B}_{z1}B_{y1}B^{2}_{y2}e^{-i\zeta_{z1}}\tilde{C}_{20}) \\ +(B_{z2}B_{y1}\overline{B}_{y2}e^{-i\zeta_{z1}}\tilde{C}_{20} - 2\omega_{1}\omega_{2}\overline{B}_{z1}\overline{B}_{y1}B_{y2}e^{-(-\zeta_{z1}-\zeta_{y1}+\zeta_{y2})}\tilde{C}_{17} + \omega_{1}^{2}B_{z2}\overline{B}_{y1}B^{2}_{1}e^{-(\zeta_{z1}-2\zeta_{y1})}\tilde{C}_{18} \\ -2\tilde{G}_{1}\omega_{2}^{2}B_{$$

The secular terms for  $\omega_2$ ,  $3\omega_1$  harmonics of the *z*-dir and  $2^{nd}$  mode are:

$$-2(i\omega_{2}B'_{z2}e^{-i\varsigma_{z2}} + \omega_{2}\varsigma'_{z2}B_{z2}e^{-i\varsigma_{z2}}) - (c_{z} - \hat{a}_{AIU})i\omega_{2}B_{z2}e^{-i\varsigma_{z2}} - B_{z1}^{3}e^{-3i\varsigma_{z1}}\tilde{C}_{8} - 2B_{z1}\bar{B}_{z1}B_{z2}e^{-i\varsigma_{z2}}\tilde{C}_{9} - 3B_{z2}^{2}\bar{B}_{z2}e^{-i\varsigma_{z2}}\tilde{C}_{10} \\ -B_{z1}B_{y1}^{2}e^{-i(\varsigma_{z1}+2\varsigma_{y1})}\tilde{C}_{8} - (B_{z1}\bar{B}_{y1}B_{y2}e^{-i(\varsigma_{z1}-\varsigma_{y1}+\varsigma_{y2})} + \bar{B}_{z1}B_{y1}B_{y2}e^{-i(-\varsigma_{z1}+\varsigma_{y1}+\varsigma_{y2})})\tilde{C}_{11} - 2B_{z2}B_{y1}\bar{B}_{y1}e^{-i\varsigma_{z2}}\tilde{C}_{12} \\ -(2B_{z2}B_{y2}\bar{B}_{y2}e^{-i\varsigma_{z2}} + \bar{B}_{z2}B_{y2}^{2}e^{-i(-\varsigma_{z2}+2\varsigma_{y2})})\tilde{C}_{10} + \omega_{1}^{2}[B_{z1}^{3}e^{-3i\varsigma_{z1}}\tilde{C}_{21} + 2B_{z1}\bar{B}_{z1}B_{z2}e^{-i\varsigma_{z2}}(\tilde{C}_{22}+\tilde{C}_{23})] \\ +\omega_{2}^{2}[2B_{z1}\bar{B}_{z1}B_{z2}e^{-i\varsigma_{z2}}\tilde{C}_{22} + 3B_{z2}^{2}\bar{B}_{z2}e^{-i\varsigma_{z2}}\tilde{C}_{24}] + \omega_{1}^{2}B_{z1}^{3}e^{-3i\varsigma_{z1}}\tilde{C}_{21} - 2\omega_{1}^{2}B_{z1}\bar{B}_{z2}e^{-i\varsigma_{z2}}\tilde{C}_{23} - \omega_{1}^{2}B_{z2}^{2}\bar{B}_{z2}e^{-i\varsigma_{z2}}\tilde{C}_{24} \\ +\omega_{1}^{2}[B_{z1}B_{y1}^{2}e^{-i(\varsigma_{z1}+2\varsigma_{y1})}\tilde{C}_{21} + (B_{z1}\bar{B}_{y1}B_{y2}e^{-i(\varsigma_{z1}-\varsigma_{y1}+\varsigma_{y2})} + \bar{B}_{z1}B_{y1}B_{y2}e^{-i(-\varsigma_{z1}+\varsigma_{y1}+\varsigma_{y2})})\tilde{C}_{22} + 2B_{z2}B_{y1}\bar{B}_{y1}e^{-i\varsigma_{z2}}\tilde{C}_{23}] \\ -2\omega_{1}^{2}B_{z2}B_{y1}\bar{B}_{y1}e^{-i\varsigma_{z2}}\tilde{C}_{23} + \omega_{2}^{2}[(B_{z1}\bar{B}_{y1}B_{y2}e^{-i(\varsigma_{z1}-\varsigma_{y1}+\varsigma_{y2})} + \bar{B}_{z1}B_{y1}B_{y2}e^{-i(-\varsigma_{z1}+\varsigma_{y1}+\varsigma_{y2})})\tilde{C}_{22} \\ +(2B_{z2}B_{y2}\bar{B}_{y2}e^{-i\varsigma_{z2}} + \bar{B}_{z2}B_{y2}^{2}e^{-i(-\varsigma_{z2}+2\varsigma_{y2})})\tilde{C}_{24}] + \omega_{1}^{2}B_{z1}B_{y1}^{2}e^{-i(-\varsigma_{z1}+\varsigma_{y1}+\varsigma_{y2})}\tilde{C}_{21} \\ +2\omega_{1}\omega_{2}(-B_{z1}\bar{B}_{y1}B_{y2}e^{-i(-\varsigma_{z1}+\varsigma_{y1}+\varsigma_{y2})})\tilde{C}_{24} + \bar{B}_{z1}B_{y1}B_{y2}e^{-i(-\varsigma_{z1}+2\varsigma_{y1})}\tilde{C}_{21} \\ +\omega_{2}^{2}(-2B_{z2}B_{y2}\bar{B}_{y2}e^{-i(\varsigma_{z1}} + \bar{B}_{z1}B_{y1}B_{y2}e^{-i(-\varsigma_{z1}+\varsigma_{y1}+\varsigma_{y2})})\tilde{C}_{22} \\ +\omega_{2}^{2}(-2B_{z2}B_{y2}\bar{B}_{y2}e^{-i(\varsigma_{z1}} + \bar{B}_{z2}B_{y2}^{2}e^{-i(-\varsigma_{z2}+2\varsigma_{y2})})\tilde{C}_{24} + \bar{q}_{z2}e^{i\sigma\pi_{1}} \\ +\omega_{2}^{2}(-2B_{z2}B_{y2}\bar{B}_{y2}e^{-i(\varsigma_{z2}} + \bar{B}_{z2}B_{y2}^{2}e^{-i(-\varsigma_{z2}+2\varsigma_{y2})})\tilde{C}_{24} + \bar{q}_{z2}e^{i\sigma\pi_{1}} \\ +\omega_{2}^{2}(-2B_{z2}B_{y2}\bar{B}_{y2}e^{-i(-\varsigma_{z2}} + \bar{B}_{z2}}B_{y2}^{2}e^{-i(-\varsigma_{z2$$

where

$$\tilde{G}_m = \frac{1}{\int_0^1 \varphi_m^2 dx}, \ (m = 1, 2)$$
(A1)

$$\tilde{q}_{m} = \frac{q_{m} \int_{0}^{1} \varphi_{m} dx}{\int_{0}^{1} \varphi_{m}^{2} dx}, \quad (m = 1, 2)$$
(A2)

$$\tilde{C}_{1} = \tilde{G}_{1} \int_{0}^{1} \varphi_{1} (\varphi_{1}^{"'''} + 4\varphi_{1}^{'} \varphi_{1}^{"''} + \varphi_{1}^{'2} \varphi_{1}^{iv}) dx$$
(A3)

$$\tilde{C}_{2} = \tilde{G}_{1} \int_{0}^{1} \varphi_{1} (3\varphi_{1}^{''2}\varphi_{2}^{''} + 4\varphi_{1}^{\prime}\varphi_{1}^{''}\varphi_{2}^{'''} + 4\varphi_{2}^{\prime}\varphi_{1}^{''}\varphi_{1}^{'''} + \varphi_{2}^{\prime'2}\varphi_{1}^{''}\varphi_{2}^{''} + 2\varphi_{1}^{\prime}\varphi_{2}^{\prime}\varphi_{1}^{''})dx$$
(A4)

$$\tilde{C}_{3} = \tilde{G}_{1} \int_{0}^{1} \varphi_{1} (3\varphi_{1}'' \varphi_{2}''' + 4\varphi_{1}' \varphi_{2}'' \varphi_{1}''' + 4\varphi_{2}' \varphi_{1}'' \varphi_{2}''' + 4\varphi_{2}' \varphi_{1}'' \varphi_{2}'' \varphi_{1}'' + 2\varphi_{1}' \varphi_{2}' \varphi_{2}'' + \varphi_{2}'^{2} \varphi_{1}^{iv}) dx$$
(A5)

$$\tilde{C}_{4} = \tilde{G}_{1} \int_{0}^{1} \varphi_{1} (2\varphi_{1}^{"2}\varphi_{2}^{"} + 4\varphi_{1}^{'}\varphi_{1}^{"}\varphi_{2}^{"} + \varphi_{1}^{'2}\varphi_{2}^{iv} + 3\varphi_{1}^{'}\varphi_{2}^{'}\varphi_{1}^{"''} + \varphi_{1}^{"}\varphi_{2}^{'}\varphi_{1}^{"''} + \varphi_{1}^{'}\varphi_{2}^{'}\varphi_{1}^{iv})dx$$
(A6)

$$\tilde{C}_{5} = \tilde{G}_{1} \int_{0}^{1} \varphi_{1}(\varphi_{1}^{"'2} \varphi_{2}^{"} + 3\varphi_{2}^{'} \varphi_{1}^{"} \varphi_{1}^{"''} + \varphi_{2}^{'} \varphi_{1}^{'} \varphi_{1}^{'''} + \varphi_{1}^{'} \varphi_{2}^{'} \varphi_{1}^{iv}) dx$$
(A7)

$$\tilde{C}_{6} = \tilde{G}_{1} \int_{0}^{1} \varphi_{1}(\varphi_{1}'' \varphi_{2}''' + 3\varphi_{1}' \varphi_{2}' \varphi_{2}''' + \varphi_{1}'' \varphi_{2}' \varphi_{2}''' + \varphi_{1}' \varphi_{2}' \varphi_{2}^{iv}) dx$$
(A8)

$$\tilde{C}_{7} = \tilde{G}_{1} \int_{0}^{1} \varphi_{1} (2\varphi_{1}'' \varphi_{2}''' + 3\varphi_{2}' \varphi_{1}'' \varphi_{2}''' + \varphi_{1}' \varphi_{2}' \varphi_{2}^{iv} + 4\varphi_{2}' \varphi_{2}'' \varphi_{1}''' + \varphi_{2}'^{2} \varphi_{1}^{iv}) dx$$
(A9)

$$\tilde{C}_8 = \tilde{G}_2 \int_0^1 \varphi_2 (\varphi_1''^3 + 4\varphi_1' \varphi_1'' \varphi_1''' + \varphi_1'^2 \varphi_1^{i\nu}) dx$$
(A10)

$$\tilde{C}_{9} = \tilde{G}_{2} \int_{0}^{1} \varphi_{2} (3\varphi_{1}^{''2}\varphi_{2}^{''} + 4\varphi_{1}^{\prime}\varphi_{1}^{''}\varphi_{2}^{'''} + 4\varphi_{1}^{\prime}\varphi_{2}^{\prime}\varphi_{1}^{'''} + 4\varphi_{2}^{\prime}\varphi_{1}^{\prime}\varphi_{2}^{'''} + \varphi_{2}^{\prime}\varphi_{2}^{\prime}\varphi_{1}^{'''} + 2\varphi_{1}^{\prime}\varphi_{2}^{\prime}\varphi_{1}^{''})dx$$
(A11)

$$\tilde{C}_{10} = \tilde{G}_2 \int_0^1 \varphi_2(\varphi_2'''^3 + 4\varphi_2' \varphi_2''' + \varphi_2'^2 \varphi_2^{i\nu}) dx$$
(A12)

$$\tilde{C}_{11} = \tilde{G}_2 \int_0^1 \varphi_2 (2\varphi_1''^2 \varphi_2'' + 4\varphi_1' \varphi_1'' \varphi_2''' + \varphi_1'^2 \varphi_2^{i\nu} + 3\varphi_1' \varphi_2'' \varphi_1''' + \varphi_1' \varphi_2' \varphi_1''' + \varphi_1' \varphi_2' \varphi_1^{i\nu}) dx$$
(A13)

$$\tilde{C}_{12} = \tilde{G}_2 \int_0^1 \varphi_2(\varphi_1''^2 \varphi_2'' + 3\varphi_2' \varphi_1'' \varphi_1''' + \varphi_2'' \varphi_1' \varphi_1''' + \varphi_1' \varphi_2' \varphi_1^{i\nu}) dx$$
(A14)

$$\tilde{C}_{13} = \tilde{G}_1 \int_0^1 \varphi_1(\varphi_2'''^3 + 4\varphi_2' \varphi_2''' \varphi_2''' + \varphi_2'^2 \varphi_2^{i\nu}) dx$$
(A15)

$$\tilde{C}_{15} = \tilde{G}_2 \int_0^1 \varphi_2 (\varphi_1'' \varphi_2'''^2 + 3\varphi_2' \varphi_1'' \varphi_2''' + \varphi_2'' \varphi_1' \varphi_2''' + 4\varphi_2' \varphi_2'' \varphi_1''' + \varphi_1' \varphi_2' \varphi_2^{i\nu} + \varphi_2'^2 \varphi_1^{i\nu}) dx$$
(A17)

$$\tilde{C}_{16} = \tilde{G}_1 \int_0^1 \varphi_1 \sum_{i=1}^2 \left[ \varphi_1'' \int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1'^2 dx_i dx_i + \varphi_1' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1'^2 dx_i dx_i)' \right] dx$$
(A18)

$$\tilde{C}_{17} = \tilde{G}_1 \int_0^1 \varphi_1 \sum_{i=1}^2 \left[ \varphi_1'' \int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1' \varphi_2' dx_i dx_i + \varphi_1' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1' \varphi_2' dx_i dx_i)' \right] dx$$
(A19)

$$\tilde{C}_{18} = \tilde{G}_1 \int_0^1 \varphi_1 \sum_{i=1}^2 \left[ \varphi_2'' \int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1'^2 dx_i dx_i + \varphi_2' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1'^2 dx_i dx_i)' \right] dx$$
(A20)

$$\tilde{C}_{19} = \tilde{G}_1 \int_0^1 \varphi_1 \sum_{i=1}^2 \left[ \varphi_2'' \int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1' \varphi_2' dx_i dx_i + \varphi_2' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1' \varphi_2' dx_i dx_i)' \right] dx$$
(A21)

$$\tilde{C}_{20} = \tilde{G}_1 \int_0^1 \varphi_1 \sum_{i=1}^2 \left[ \varphi_1'' \int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_2'^2 dx_i dx_i + \varphi_1' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_2'^2 dx_i dx_i)' \right] dx$$
(A22)

$$\tilde{C}_{21} = \tilde{G}_2 \int_0^1 \varphi_2 \sum_{i=1}^2 \left[ \varphi_1'' \int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1'^2 dx_i dx_i + \varphi_1' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1'^2 dx_i dx_i)' \right] dx$$
(A23)

$$\tilde{C}_{22} = \tilde{G}_2 \int_0^1 \varphi_2 \sum_{i=1}^2 \left[ \varphi_1'' \int_{l_i}^{x_i} \varphi_1' \varphi_2' dx_i dx_i + \varphi_1' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1' \varphi_2' dx_i dx_i)' \right] dx$$
(A24)

$$\tilde{C}_{23} = \tilde{G}_2 \int_0^1 \varphi_2 \sum_{i=1}^2 \left[ \varphi_2'' \int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1'^2 dx_i dx_i + \varphi_2' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1'^2 dx_i dx_i)' \right] dx$$
(A25)

$$\tilde{C}_{24} = \tilde{G}_2 \int_0^1 \varphi_2 \sum_{i=1}^{2} \left[ \varphi_2'' \int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_2'^2 dx_i dx_i + \varphi_2' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_2'^2 dx_i dx_i)' \right] dx$$
(A26)

$$\tilde{C}_{25} = \tilde{G}_2 \int_0^1 \varphi_1 \sum_{i=1}^2 \left[ \varphi_2'' \int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_2'^2 dx_i dx_i + \varphi_2' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_2'^2 dx_i dx_i)' \right] dx$$
(A27)

$$\tilde{C}_{26} = \tilde{G}_2 \int_0^1 \varphi_2 \sum_{i=1}^2 \left[ \varphi_2'' \int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1' \varphi_2' dx_i dx_i + \varphi_2' (\int_{l_i}^{x_i} \int_{l_{i-1}}^{x_i} \varphi_1' \varphi_2' dx_i dx_i)' \right] dx$$
(A28)

## References

- [1] E. Özkaya, "Non-linear transverse vibrations of a simply supported beam carrying concentrated masses," Journal of Sound and Vibration, vol. 257, no. 3, pp. 413-424, October 2002.
- [2] J. S. Mundrey, Railway track engineering, Tata McGraw-Hill, 2000.
- [3] T. Phuoc Nguyen, D. Trung Pham, and P. Hoa Hoang, "Effects of foundation mass on dynamic responses of beams subjected to moving oscillators," Journal of Vibroengineering, Vol. 22, no. 2, pp. 280-297, March 2020.
- [4] T. P. Chang, "Nonlinear free vibration analysis of nano-beams under magnetic field based on non-local elasticity theory," Journal of Vibroengineering, vol. 18, no. 3, pp. 1912-1919, May 2016.
- [5] Z. Zhang, S. R. Nielsen, F. Blaabjerg, and D. Zhou, "Dynamics and control of lateral tower vibrations in offshore wind turbines by means of active generator torque," Energies, vol. 7, no. 11, pp. 7746-7772, November 2014.
- [6] A. H. Nayfeh and D. T. Mook, Nonlinear oscillations, Wiley-Interscience Publication, New York, pp.95-160, 1995.

- [7] P. F. Pai, "Nonlinear flexural-flexural-torsional dynamics of metallic and composite beams," Ph.D. dissertation, Department of Engineering Science and Mechanics, Virginia Polytechnic Institute and State University, 1990.
- [8] S. Stoykov and P. Ribeiro, "Stability of nonlinear periodic vibrations of 3D beams," Nonlinear Dynamics, vol. 66, no. 3, pp. 335, August 2011.
- [9] W. T. van Horssen and G. J. Boertjens, "On mode interactions for a weakly nonlinear beam equation," Nonlinear Dynamics, vol. 17, no. 1, pp. 23-40, September 1998.
- [10] W. T. van Horssen and G. J. Boertjens, "An asymptotic theory for a weakly nonlinear beam equation with a quadratic perturbation," SIAM Journal on Applied Mathematics, vol. 60, no. 2, pp. 602-632, 2000.
- [11] Y. R. Wang, C. K. Feng, and S. Y. Chen, "Damping effects of linear and nonlinear tuned mass dampers on nonlinear hinged-hinged beam," Journal of Sound and Vibration, vol. 430, pp. 150-173, 2018.
- [12] Y. R. Wang and W. C. Hsiao, "Vibration reduction of damping rings on 3D nonlinear multi-loaded slender beams," Journal of Chinese Society of Mechanical Engineers, vol. 40, no.4, pp. 327-339, June 2019.
- [13] A. Tekin, E. Özkaya, and S. M. Bağdatlı, "Three-to-one internal resonance in multiple stepped beam systems," Applied Mathematics and Mechanics, vol. 30, no. 9, pp. 1131-1142, September 2009.
- [14] A. H. Nayfeh and P. F. Pai, Linear and nonlinear structural mechanics, New York: John Wiley & Sons, 2004.
- [15] Z. Ji and J. W. Zu, "Method of multiple scales for vibration analysis of rotor-shaft systems with non-linear bearing pedestal model," Journal of Sound and Vibration, vol. 218, no. 2, pp. 293-305, November 1998.
- [16] A. H. Nayfeh and S. A. Nayfeh, "On nonlinear modes of continuous systems," Journal of Vibration and Acoustics, vol. 116, no. 1, pp. 129-136, January 1994.
- [17] Q. Mao, "Free vibration analysis of multiple-stepped beams by using Adomian decomposition method," Mathematical and Computer Modeling, vol. 54, no.1-2, pp. 756-764, March 2011.
- [18] D. Jeyakumar, K. K. Blswas, and B. Nageswara Rao, "Stage separation dynamic analysis of upper stage of a multistage launch," Mathematical and Computer Modeling, vol. 41, no. 8-9, pp. 849-866, April-May 2005.
- [19] G. Srinivas and M. V. S. Prakash, "Aerodynamics and flow characterization of multistage rockets," IOP Conference Series: Materials Science and Engineering, vol. 197, no. 1, article number 012077, July 2017.
- [20] W. T. van Horssen, "An asymptotic theory for a class of initial-boundary value problems for weakly nonlinear wave equations with an application to a model of the galloping oscillations of overhead transmission lines," SIAM Journal of Applied Mathematics, vol. 48, no. 6, pp. 1227-1243, 1988.
- [21] "WayBack Machine," https://web.archive.org/web/20150824115334/.
- [22] "European Space Agency," http://wsn.spaceflight.esa.int/docs/EUG2LGPr3/EUG2LGPr3-6-SoundingRockets.pdf/.
- [23] K. Torabi, H. Afshari, and H. Najafi, "Vibration analysis of multi-step Bernoulli-Euler and Timoshenko beams carrying concentrated masses," Journal of Solid Mechanics, vol. 5, no. 4, pp. 336-349, 2013.



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